

UNIVERSITI PUTRA MALAYSIA

EFFECTS OF SUSPENDED AND UNSUSPENDED ADDITIVES IN SHORT-TERM AGING PROCESS OF BINDER

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EFFECTS OF SUSPENDED AND UNSUSPENDED ADDITIVES IN SHORT-TERM AGING PROCESS OF BINDER

By

TAHSEEN SAAD ALI

Thesis Submitted to the School of Graduation Studies, Universiti Putra Malaysia, in Fulfillment of the Requirements for the Degree of Master of Science

April 2014

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Abstract of the thesis presented to the senate of Universiti Putra Malaysia in fulfillment of the requirement for the degree of Master of Science.

EFFECTS OF SUSPENDED AND UNSUSPENDED ADDITIVES IN SHORT-TERM AGING PROCESS OF BINDER

By

TAHSEEN SAAD ALI

April 2014

Chairman: Prof. Ratnasamy Muniandy, Phd

Faculty: Engineering

Recently, numerous additives have been blended in asphalt binder to enhance the performance of pavement mixture. However, the use of these modified binders can be problematic as their outcomes on site and in road construction are unpredictable. Meanwhile, many stick to using the same known additives because they are tested and the results predictable and do not experiment with new and possibly better alternatives that could yield improved results. The poor correlation of fatigue and rutting potentials of asphalt binder with the asphalt mix is the main concern in this research.

Modified asphalt properties are different in specifications according to the additive type and behavior within the modified binder and as a result of this, there is a fear of unpredictable pavement failure. Such a passive attitude will mean ignoring the potential advantages of the asphalt concrete mix in favor of the asphalt binder which faces problems of rutting and fatigue. Accordingly, the additives are classified into two groups: non-suspended and suspended modified binders. Moreover, the suspended additives continue to be used and the inherent problems remain. These two groups have been compared as they are totally different in their physical properties – one melts while the other is suspended within the binder formation.

This study evaluates the non-suspended and suspended additives. Each group consists of different types of additives. Group one (melted) is divided into two additives, ethylene vinyl acetate and tire rubber powder. Group two (suspended) consists of hydrated mineral filler and oil palm cellulose fiber.

The physical tests penetration and softening point and the superpave test represents rotational viscosity, rolling thin film oven test RTFO for short-term aging and lastly DSR which are used to evaluate the non-suspended and suspended additives. The main tool for group evaluation is conducted by using the dissipated energy approach to diagnose the failure. The stress sweep test is chosen to run the DSR test.

The results reveal the superior resistance to the shear resistance that is represented by the non-suspended additives modified binders. The limit state is established for the

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non-suspended additives especially for the NSEU (2%-8%) and NSEA (4%-8%). Lastly, the non-suspended modified binders reduce the rutting problems more than the suspended additives modified binders.



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KESAN BAGI BAHAN TAMBAH TERAMPAI DAN TIDAK TERAMPAI DALAM PROSESPENUAAN JANGKA PENDEK BAGI BAHAN PENGIKAT

Oleh

TAHSEEN SAAD ALI

April 2014

Pengerusi: Prof. Ratnasamy Muniandy, PhD Fakulti: Kejuruteraan

Sejak kebelakangan ini, pelbagai bahan tambah telah dicampurkan dalam pengikat aspal untuk meningkatkan prestasi bagi turapan campuran. Walaubagaimanapun, penggunaan bagi pengikat yang telah diubahsuai akan menjadi satu masalah kerana dapatan mereka keatas tapak dan dalam pembinaan jalan adalah diluar jangka. Sementara itu, ramai pihak yang masih menggunakan bahan tambah yang sama kerana ianya telah diuji dan hasilnya dapat dijangka dan tidak mencuba yang baru dan mungkin alternatif yang lebih baik yang mana mampu menghasilkan dapatan yang lebih baik. Korelasi yang tidak sesuai bagi kemungkinan bagi kelusuhan dan pengeluman bagi pengikat aspal dengan campuran aspal adalah fokus utama dalam kajian ini.

Sifat aspal yang diubahsuai adalah sangat berbeza dalam spesifikasi berdasarkan kepada jenis dan sifat dalam pengikat ubahsuai dan sebagai hasilnya, terdapatnya kegusaran berkenaan kegagalan dalam turapan yang tidak terjangka. Sikap pasif sebegini bermaksud tidak menghiraukan kelebihan yang berpotensi bagi campuran konkrit aspal daripada pengikat aspal yang mana menghadapi masalah kelusuhan dan pengeluman Secara berasingan, bahan penambah telah dikumpulkan kepada dua kumpulan iaitu tidak terampai dan pengikat ubahsuai terampai. Tambahan lagi, bahan penambah terampai berterusan digunakan dan masalah yang seringkali dihadapi berulang. Kedua-dua kumpulan ini telah dibandingkan dan mereka benarberna berbeza dalam sifat fizikal mereka – satu cair manakala satu yang lain terampai dalam formasi pengikat.

Kajian ini menilai bahan tambah yang tidak terampai dan terampai. Setiap kumpulan mengandungi jenis bahan tambah yang berbeza. Kumpulan satu (cair) telah diasingkan kepada dua bahan tambah, etilena vinil asetat dan serbuk tayar getah. Kumpulan kedua (terampai) mengandungi penapis mineral yang hidrat dan fiber selulosa minyak kelapa sawit.

Ujian fizikal penebusan dan poin kelembutan dan ujian 'superpave' mewakili kelikatan berputar, ujian filem oven nipis berputar RKTO bagi penuaan jangka masa pendek dan akhirnya DSR yang mana telah digunakan bagi menilai bahan tambah



tidak terampai dan terampai. Peralatan utama bagi penilaian kumpulan telah dijalankan dengan menggunakan pendekatan tenaga 'terlesap' untuk mendiagnosis kegagalan. Mod kawalan stress telah dipilih untuk menjalankan ujian DSR.

Dapatan kajian menunjukkan rintangan yang besar kepada rintangan ricih yang diwakili oleh bahan tambah tidak terampai yang diubahsuai pengikat. Tahap had telah dikuatkuasakan bagi bahan tambah tidak terampai terutamanya bagi NSEU (2%-8%) dan NSEA (4%-8%). Akhir sekali, bahan tambah tidak terampai pengikat ubahsuai mengurangkan masalah pengeluman lebih daripada bahan tambah terampai pengikat ubahsuai.

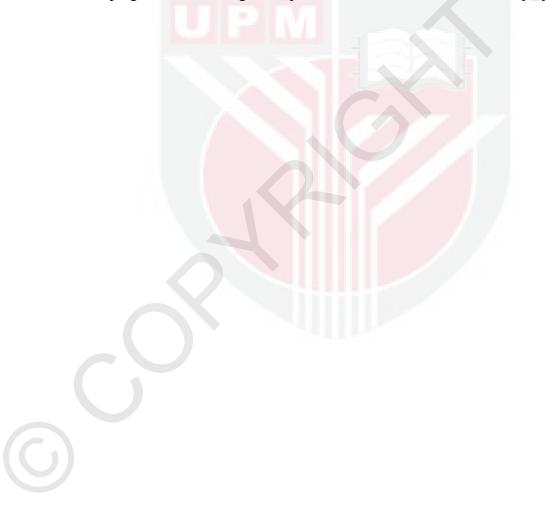
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This thesis was submitted to the Senate of Universiti Putra Malaysia and has been accepted as fulfillment of the requirement for the degree of Master of Science. The members of the Supervisory Committee were as follows:

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Declaration by the student

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	PhD –		

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C

LIST OF ABBREVIATIONS

AC :- Asphalt cement		
AASHTO: - American Association of State Highways and Transportation Officials		
ASTM : - American Society for Testing and Materials		
ANN : - Artificial neural network		
ANOVA: - analysis-of-variance		
BTD : - Bareclona Traccion Directa		
CRM : - Crumb rubber modified		
DER : - Dissipated Energy Ratio		
DMA : - Dynamic mechanical analysis		
DSC : -Differential scanning calorimetry		
DTT : - Direct tensile test		
DPSE :-Dissipated pseudostrain energy		
EBA :-Ethylene butyle acrylate		
EVA :-Ethylene vinyl acetate		
FAM :-Fine aggregate matrix		
HDPE :-High density polyethylene		
HMA :-Hot mix asphalt		
HMF : -Hydrated mineral filler		
LDPE :-Low density polyethylene		
LVR :-Linear viscoelastic range		
NS : - non-suspended		
NSEA : - non-suspended (ethylene vinyl acetate) aged		
NSEU :- non-suspended (ethylene vinyl acetate) unaged		
NSRA :- non-suspended (tire rubber powder) aged		
NSRU : - non-suspended (tire rubber powder) unaged		
COPF : - Cellulose oil palm fiber		

PAV :-Pressure aging vessel

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- PSI :-Percent severability index
- PV :-Plateau value
- RDEC:-Ratio dissipated energy change
- RTFOT:-Rolling thin film oven test
- RP :-Rest period
- RV :-Rotational viscometer
- RAP :-Recycle asphalt pavement
- S :- Suspended
- SBS :-Styrene-butadiene-styrene
- SEBS:-Styrene-ethylene-butadiene-styrene
- SHRP:-Strategic highway research porogram
- SMA:-Stone mastic asphalt
- TEM:-Transmission electron microscopy
- TFOT:-Thin film oven test
- TI :-Thickness index
- TSR :-Tensile strength ratio
- UCL:-Universal decaracterization de Ligantes

CHAPTER 1

INTRODUCTION

1.1 Background

Asphalt binder has been a successful material to bind the aggregate particle and to make together the asphalt concrete. In the recent century, transportation has seen a lot of development particularly since War World II. Cities have become bigger due to significant population increase. Such development has had a great impact on transportation with subsequent implications on roads and road networks.

In addition, the improvement of transportation has been accompanied by major problems on the highways. In the early years of the automotive industry, the load applied on the pavement was low even and designers did not have to contend with many problems affecting the pavement at that time. The problems started with the increasing traffic volume that subjected existing pavements to high loads. Since then, the challenge faced by researchers has been to prevent distress on the pavements or at least increase the maintenance period and both are controlled by the cost.

The major factors which affect pavement performance are the load and the environmental conditions, which lead to two types of pavement deformation permanent deformation (rutting), and fatigue cracks, rutting occurs due to high temperature and the traffic load that the pavement surface is subjected to. Fatigue cracks are the result of low temperatures and the repetitive load on the pavement surface.

It can therefore be concluded that pavement deformations are caused by either environmental factors or traffic load and most of the time both contribute to deform the pavement surface. Before War World II, simple designs did exist, but after the war there was rapid development in transportation that led to the emergence of road pavement failure. Efforts to address this problem are still going on The aim of this study will be to investigate the factors related to failure in pavements under climatic and load conditions and make recommendations for solution.

1.2 Problem Statement

Nowadays, the world has encountered a problem of high petrol prices and in general, it has affected highway construction. In other words, the cost factor increases with increasing demand for paving roads and highways. This study will therefore also consider the related economic problems.

(Johnson, Bahia, & We, 2007, and Johnson, Bahia, & Cppnen, 2009) insisted there is no correlation between the fatigue in the asphalt binder and that in the mix although different binders ranking under several load factors were considered. After Johnson (Zhou, Mogawer, & Li, 2012) the binder fatigue parameter of superpave has poor relationship with the asphalt mix fatigue based on test procedure conducted. This discussion begins with the hypothesis of how additives behave differently within the binder composition, which can be summarized in two mechanisms: non-suspended (NS) and suspended (S) additives. Based on the shear resistance measured, the hypothesis is non-suspended solid additives have increased shear resistance as compared with suspended solid additives.

When suspended additives are added to the binder, they stay within the binder combination as solid partials and no melting or dissolving occurs and this is defined as a suspended mechanism approach. On the other hand, dissolved additives melt within the binder composition at high temperatures. As such, at low temperatures they may not stay in a homogenous form, especially within the asphaltic mixture combination. Furthermore, the modifier may undergo non-homogeneity behavior which reveals a lack of distribution of the modifier within the binder's molecular structure, which is called melted mechanism approach. Such a hypothesis may have a distinct effect on the unclear relation between fatigue and rutting in the binder and in the mixture. In other words, when the modified binders are tested under fatigue and rutting conditions, it shows extreme results. In the field, when the same modified binders are utilized to produce the mix design, they do not reveal the same indicated property obtained from the previous modified binders test. The problems are still occurring in the pavement despite various types of additives being used so far.

The aging terms definitions begin from the short term to the long term. At the plant mix while the mix is prepared the asphalt film thickness will be under two affecting factors of temperature and time of the mix; the higher the temperature and the longer the delaying time of the mix the higher the aging will be and this is referred to as short-term aging and it is due to volatilization.

Asphalt samples taken from a project completed for a certain period of time were subjected to laboratory tests to recover the asphalt binder and it was found that there was an increase in the asphalt binder's viscosity compared to the initial viscosity. The increased of viscosity was due to oxidation of the asphalt binder since the pavement was compacted till the coring sample and the laboratory tests is defined as long-term aging (Bell, 1989). The ultraviolet is also considered one of the factors involved in the aging phenomenon that researchers have been concerned about (Wu, Pang, Liu, & Zhu, 2010).

The quite common distress in asphaltic concrete such as, fatigue cracks and rutting, is caused by aging of the asphalt which increases the viscosity and then, the asphalt stiffness will increase at low temperature which makes the asphalt concrete prone to cracks. (Robertson, 1991) clarified that high and low temperatures have a damaging effect and contributes to the aging phenomenon to produce the distress in pavement construction. At low temperatures, the asphalt binder becomes brittle and then, the fatigue problem occurs. At high temperatures, the asphalt binder becomes soft while the high load stress generated by the traffic movement causes rutting as shown in Figure 1.1 and the final findings are tabulated in Table 1.1

PERFORMANCE

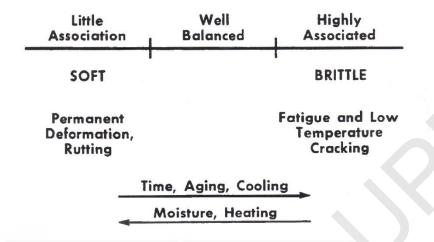


Figure 1.1: Illustrating of various pavement distresses under the effect of temperature, time and oxidation (Robertson et al., 1991).

Table 1.1: The summary of findings for the problem statement.

Source	Problems
Zhou et al., 2013	poor relationship between the fatigue of binder & AC
Johnson et al., 2007, 2009	Fatigue and rutting correlation with asphalt mix
Bell, 1989	Long and short- term aging progress in asphalt binder
Robertson, 1991	High and low temperature problem

1.3 Objectives of Study

The rheological properties are the significant part in this study. The NS and S groups can be characterize based on their concentration with the lower and upper limits needed to be added to fulfill the pavement construction neediness. Lastly, comparing the deformation techniques relationship with in the aged and unaged property for the asphalt binders for the NS and S MB groups to find the correlations.

- 1- To determine the rheological properties of modified binders.
- 2- To establish a limit state boundary for aged and unaged asphalt
- 3- To establish a correlation between the modified asphalt aged and unaged property of rutting performance of binders.

1.4 The importance of the Study

In the recent years, the asphalt binders are modified with numerous types of asphalt. Currently, those types are classified based on their blending mechanism. They are named non-suspended (NS) and suspended (S). for example, polymers and rubber characterized as NS and cellulose fibers and mineral filler characterized as S.

The DSR has been widely used to evaluate the rheological properties, as it is used to measure fatigue and rutting potentials of bituminous materials. The non-suspended and suspended groups are evaluated in DSR to find which group is more compatible with such important equipment.

Finally, the compatibility of the additive types used in the current research with the 80/100 penetration grade binder. This is highlighted in the literature that some of the binders and their modifiers did not meet significant results due to incompatibility.

1.5 The scope of the study

There are a huge number of additives used to modified the asphalt binders. In general, they blend in two blending mechanisms non-suspended solid and suspended solid groups. Independently, each group has a huge numbers of additives. In the current research, the non-suspended additives include polymer EVA and tire rubber powder which they are wildly used in the pavement construction as well as most of the researchers studied their properties. The suspended additives include the oil palm cellulose fiber and the mineral filler due to their usage which is lowering the drain down and induce moisture damage agent and also due to their common use in the pavement construction.

REFERENCES

- Airey, G. D. (2002). Rheological evaluation of ethylene vinyl acetate polymer modified bitumens. *Construction and Building Materials*, 16(8), 473-487.
- Airey, G. D., & Rahimzadeh, B. (2004). Combined bituminous binder and mixture linear rheological properties. *Construction and Building Materials*, 18(7), 535-548.
- Airey, G. D., Rahimzadeh, B., & Collop, A. C. (2003). Viscoelastic linearity limits for bituminous materials. *Materials and Structures*, *36*(10), 643-647.
- Airey, G. D., Rahimzadeh, B., & Collop, A. C. (2004). Linear rheological behavior of bituminous paving materials. *Journal of Materials in Civil Engineering*, 16, 212.
- Al-Hadidy, A., & Yi-qiu, T. (2009). Effect of polyethylene on life of flexible pavements. *Construction and Building Materials*, 23(3), 1456-1464.
- Ali, B., & Sadek, M. (2011). Experimental analysis of the influence of crumb rubber addition on the short-term aging of syrian asphalt. *Arabian Journal of Geosciences*, , 1-6.
- Anderson, D. A., Hir, Y. M. L., Marasteanu, M. O., Planche, J. P., Martin, D., & Gauthier, G. (2001). Evaluation of fatigue criteria for asphalt binders. *Transportation Research Record: Journal of the Transportation Research Board*, 1766(-1), 48-56.
- Anderson, D. (1987). Construction, operation, and application of the FHWA Accelerated Loading Facility. ..., 2nd, 1987, Toronto Retrieved from http://pavementmanagement.org/ICMPfiles/1987073.pdf
- Anderson, D. A., Christensen, D. W., Bahia, H. U., Dongre, R., Sharma, M. G., Antle, C. E., & Button, J. (1994). Binder characterization and evaluation, volume 3: Physical characterization. *Strategic Highway Research Program, National Research Council, Washington, DC.*
- Anwar Parvez, M., Al-Mehthel, M., Al-Abdul Wahhab, H. I., & Hussein, I. a. (2014). Utilization of sulfur and crumb rubber in asphalt modification. *Journal of Applied Polymer Science*, 131(7), n/a–n/a. doi:10.1002/app.40046
- Bahia and Anderson D. A., H. (1993). The new propsed rheological properties of asphalt binders: why are they required and how do they compare to conventional properties. *ASTM Conference: Physical Properties of Asphalt Cement Binders, Texas*, 1–27.

- Bahia, H. U., & Anderson, D. A. (1995). 'The pressure aging vessel (PAV): a test to simulate reological changes due to field aging. *Physical Properties of Asphalt Cement Binders*, 67–88.
- Bahia, H. U., & Davies, R. (1994). Effect of crumb rubber modifiers (crm) on performance-related properties of asphalt binders (with discussion). *Journal of the Association of Asphalt Paving Technologists, 63*
- Bahia, H., Zhai, H., Onnetti, K., & Kose, S. (1999). Non-linear viscoelastic and fatigue properties of asphalt binders. *Journal of the Association of Asphalt Paving Technologists*, 68
- Bahia, H. U., Nam, K., & Delgadillo, R. (2004). Development of Guidelines for PG Binder Selection for Wisconsin. WisDOT Highway Research Study.
- Bari, J., & Witczak, M. W. (2005). Evaluation of the effect of lime modification on the dynamic modulus stiffness of hot-mix asphalt: Use with the new mechanistic-empirical pavement design guide. *Transportation Research Record: Journal of the Transportation Research Board*, 1929(1), 10–19.
- Bell, A. C., Sosnovske, D. (1994). Aging: Binder validation.
- Bhasin, A., Branco, V. T. F. C., Masad, E., & Little, D. N. (2009). Quantitative comparison of energy methods to characterize fatigue in asphalt materials. *Journal of Materials in Civil Engineering*, 21, 83.
- Billiter, T., Davison, R., Glover, C., & Bullin, J. (1997). Physical properties of asphalt-rubber binder. *Petroleum Science and Technology*, 15(3), 205-236.
- Brovelli, C., Hilliou, L., Hemar, Y., Pais, J., Pereira, P., & Crispino, M. (2013). Rheological characteristics of EVA modified bitumen and their correlations with bitumen concrete properties. *Construction and Building Materials*, 48, 1202–1208. doi:10.1016/j.conbuildmat.2013.07.032
- Carlson, D. D., & Zhu, H. (1999). Asphalt-rubber an anchor to crumb rubber markets.
- Carpenter, S. H., Ghuzlan, K. A., & Shen, S. (2003). Fatigue endurance limit for highway and airport pavements. *Transportation Research Record: Journal of the Transportation Research Board*, 1832(-1), 131-138.
- Carpenter, S. H., & Shen, S. (2006). Dissipated energy approach to study hot-mix asphalt healing in fatigue. *Transportation Research Record: Journal of the Transportation Research Board*, 1970(-1), 178-185.
- Castro, M., & Sánchez, J. A. (2006). Fatigue and healing of asphalt mixtures: Discriminate analysis of fatigue curves. *Journal of Transportation Engineering*, 132, 168.

- Chen. (1997). Rheological properties of asphalt-mineral filler mastics. No. 571/V-36, 269-277
- Chen, J. S., & Lin, K. Y. (2005). Mechanism and behavior of bitumen strength reinforcement using fibers. *Journal of Materials Science*, 40(1), 87-95.
- Chen, Z., Wu, S., Zhu, Z., & Liu, J. (2008). Experimental evaluation on high temperature rheological properties of various fiber modified asphalt binders. *Journal of Central South University of Technology*, 15, 135-139.
- Collop, A. C., Cebon, D., & Hardy, M. S. A. (1995). Viscoelastic approach to rutting in flexible pavements. *Journal of Transportation Engineering*, *121*, 82.
- Cong, P., Chen, S., Yu, J., & Wu, S. (2010). Effects of aging on the properties of modified asphalt binder with flame retardants. *Construction and Building Materials*, 24(12), 2554-2558.
- Daniel, J. S., & Kim, Y. R. (2001). Laboratory evaluation of fatigue damage and healing of asphalt mixtures. *Journal of Materials in Civil Engineering*, 13, 434.
- Dobson, G. R. (1977). Viscoelastic properties of asphalt, 36(2), 53-58.
- Feipeng, X., Wenbin, Z., & Serji, N. (2010). Aging influence on fatigue characteristics of RAC mixtures containing warm asphalt additives. Advances in Civil Engineering, 2010
- Garci'a-Morales, M., Partal, P., Navarro, F., Marti'nez-Boza, F., Gallegos, C., González, N., et al. (2004). Viscous properties and microstructure of recycled eva modified bitumen. *Fuel*, 83(1), 31-38.
- Ghaly, N. (2008). Effect of Sulfur on the Storage Stability of Tire Rubber Modified Asphalt. *World Journal of Chemistry*, 3(2), 42–50.
- Ghuzlan, K. A. (2001). Fatigue damage analysis in asphalt concrete mixtures based upon dissipated energy concept. Ph.D. thesis, Department of Civil and Environmental Engineering, University of Illinois at Urbana-Champaign, Urbana, III.
- Ghuzlan, K. A., & Carpenter, S. H. (2000). Energy-derived, damage-based failure criterion for fatigue testing. *Transportation Research Record: Journal of the Transportation Research Board*, 1723(-1), 141-149.
- Ghuzlan, K. A., & Carpenter, S. H. (2006). Fatigue damage analysis in asphalt concrete mixtures using the dissipated energy approach. *Canadian Journal of Civil Engineering*, 33(7), 890-901.
- Gorkem, C., & Sengoz, B. (2009). Predicting stripping and moisture induced damage of asphalt concrete prepared with polymer modified bitumen and hydrated lime. *Construction and Building Materials*, 23(6), 2227–2236.

- Huang, S.-C., Claine Petersen, J., Robertson, R. E., & Branthaver, J. F. (2002). Effect of hydrated lime on long-term oxidative aging characteristics of asphalt. *Transportation Research Record: Journal of the Transportation Research Board*, 1810(1), 17–24.
- Hussein, I. A., Iqbal, M. H., & Al-Abdul-Wahhab, H. I. (2005). Influence of M w of LDPE and vinyl acetate content of EVA on the rheology of polymer modified asphalt. *Rheologica acta*, 45(1), 92–104.
- Isacsson, U., & Lu, X. (1999). Characterization of bitumens modified with SEBS, EVA and EBA polymers. *Journal of Materials Science*, *34*(15), 3737–3745.
- Jafariahangari, H. (2008). Performance Of Cellulose Palm Fiber As An Additive In Asphalt Blends. Retrieved from http://psasir.upm.edu.my/5445/
- Jahromi, S. G. (2009). Estimation of resistance to moisture destruction in asphalt mixtures. *Construction and Building Materials*, 23(6), 2324–2331.
- Johnson, C. M., Bahia, H. U., & Coenen, A. (2009). Comparison of Bitumen Fatigue Testing Procedures Measured in Shear and Correlations with Four-Point Bending Mixture Fatigue. In 2nd Workshop on Four Point Bending, Guimaraes, Portugal.
- Johnson, C. M., Bahia, H. U., & Wen, H. (2007). Evaluation of Strain-Controlled Asphalt Binder Fatigue Testing in the Dynamic Shear Rheometer. In 4th international siiv congress-palermo.
- Khodary Moalla Hamed, F. (2010). Evaluation of Fatigue Resistance for Modified Asphalt Concrete Mixtures Based on Dissipated Energy Concept.
- Kim, H. S., Lee, S. J., & Amirkhanian, S. (2010). Rheology investigation of crumb rubber modified asphalt binders. *KSCE Journal of Civil Engineering*, 14(6), 839-843.
- Kim, Y. R. (2003). Fatigue and healing characterization of asphalt mixtures. *Journal* of Materials in Civil Engineering, 15, 75.
- Kim, Y. R. (2004). Linear viscoelastic analysis of asphalt mastics. *Journal of Materials in Civil Engineering*, 16, 122.
- Kök, B. V., & Iolak, H. (2011). Laboratory comparison of the crumb-rubber and SBS modified bitumen and hot mix asphalt. *Construction and Building Materials*,
- Kumar, P., Mehndiratta, H. C., & Lakshman Singh, K. (2009). Rheological properties of crumb rubber modified bitumen-A lab study. *Journal of Scientific and Industrial Research*, 68, 812–816.

Kim, Y. Richard. Modeling of asphalt concrete. McGraw-Hill: 2009.

- Kim, Y.-R., Little, D. N., & Song, I. (2003). Effect of mineral fillers on fatigue resistance and fundamental material characteristics: mechanistic evaluation. *Transportation Research Record: Journal of the Transportation Research Board*, 1832(1), 1–8.
- Lee, S. J., Amirkhanian, S. N., & Kim, K. W. (2009). Laboratory evaluation of the effects of short-term oven aging on asphalt binders in asphalt mixtures using HP-GPC. *Construction and Building Materials*, 23(9), 3087-3093.
- Lee, S. J., Amirkhanian, S. N., Shatanawi, K., & Kim, K. W. (2008). Short-term aging characterization of asphalt binders using gel permeation chromatography and selected superpave binder tests. *Construction and Building Materials*, 22(11), 2220-2227.
- Lesueur, D., & Little, D. N. (1999). Effect of hydrated lime on rheology, fracture, and aging of bitumen. *Transportation Research Record: Journal of the Transportation Research Board*, 1661(1), 93–105.
- Little, D. N., Epps, J. A., & Sebaaly, P. E. (2001). The benefits of hydrated lime in hot mix asphalt. *National Lime Association*.
- Marasteanu, M. O., & Anderson, D. A. (2000). Establishing linear viscoelastic conditions for asphalt binders. *Transportation Research Record: Journal of the Transportation Research Board*, 1728(-1), 1–6.
- Moghadas Nejad, F., Vadood, M., & Baeetabar, S. (2014). Investigating the mechanical properties of carbon fibre-reinforced asphalt concrete. *Road Materials and Pavement Design*, 15(2), 465–475. doi:10.1080/14680629.2013.876442
- Mohamed, A., Omar, H., Hamzah, M., & Ismail, H. (2009). Rheological properties of crumb rubber-modified bitumen containing antioxidant. *Arabian journal for science and engineering*, *34*(1B)
- Molenaar, A., Hagos, E., & Van de Ven, M. (2010). Effects of aging on the mechanical characteristics of bituminous binders in PAC. *Journal of Materials in Civil Engineering*, 22, 779.
- Muniandy, R., & Huat, B. B. K. (2006). Laboratory diameteral fatigue performance of stone matrix asphalt with cellulose oil palm fiber. *American Journal of Applied Sciences*, *3*(9), 2005-2010.
- Muniandy, R., & Sohadi, R. U. R. Highway materials. University putra malaysia: 2001.
- Muniandy, R. (2004). Effects of newly developed cellulose oil palm fiber in the fatigue failure of stone mastic asphalt by ratnasamy muniandy *Thesis Submitted* to the School of Graduate Studies, Universiti Putra Malaysia, in Fulfilment of the Requirements for the Degree of D. thesis. University Putra Malaysia.

- Muniandy, R., Loong, T., & Omar, H. (2001). Fatigue Modelling for Stone Mastic Asphalt (SMA). *Pertanika Journal of Science ...*, 9(1), 59–63. Retrieved from http://psasir.upm.edu.my/3635/
- Nejad, F. M., Aghajani, P., Modarres, A., & Firoozifar, H. (2011). Investigating the properties of crumb rubber modified bitumen using classic and SHRP testing methods. *Construction and Building Materials*.
- Özen, H. (2011). Rutting evaluation of hydrated lime and SBS modified asphalt mixtures for laboratory and field compacted samples. *Construction and Building Materials*, 25(2), 756–765. doi:10.1016/j.conbuildmat.2010.07.010
- Pais, J., Pereira, P., Minhoto, M., Fontes, L., Kumar, D., & Silva, B. (2009). Application of plateau value to predict fatigue life. *Proceedings of the 2nd Workshop on Four-Point Bending. University of Minho*, 59-68.
- Pell, P. (1967). Fatigue of asphalt pavement mixes. *Intl Conf Struct Design Asphalt Pvmts*,
- Plaxico, C. A., Uddin, W., & Hackett, R. M. (1996). A Micromechanical Model for Asphalt Materials. In *Materials for the New Millennium* (Vol. 1, pp. 761–770). ASCE.
- Pronk, A. C., & Hopman, P. C. (1991). Energy dissipation: the leading factor of fatigue, p. 255–267. Retrieved from http://worldcat.org/isbn/0727716352
- Putman, B. J. (2011). Effects of fiber finish on the performance of asphalt binders and mastics. *Advances in Civil Engineering*, 2011
- Raad, L., Saboundjian, S., & Minassian, G. (2001). Field aging effects on fatigue of asphalt concrete and asphalt-rubber concrete. *Transportation Research Record: Journal of the Transportation Research Board*, 1767(-1), 126-134.
- Recasens, R. M., Martínez, A., Jiménez, F. P., & Bianchetto, H. (2005). Effect of filler on the aging potential of asphalt mixtures. *Transportation Research Record: Journal of the Transportation Research Board*, 1901(1), 10–17.
- Robertson, R. E., (US), S. H. R. P., & Institute, U. of W. R. C. W. R. (1991). Chemical Properties of Asphalts and their Relationship to Pavement Performance. *Western Research Institute*.
- Roberts, F.L., Kandhal, P.S., Brown, E.R., Lee, D.Y., and Kennedy, T.W. (1996). "Hot Mix Asphalt Materials, Mixture Design, and Construction." Lanham, Maryland: NAPA Research and Education Foundation.
- Rodríguez-Alloza, A. M., Gallego, J., Pérez, I., Bonati, A., & Giuliani, F. (2014). High and low temperature properties of crumb rubber modified binders containing warm mix asphalt additives. *Construction and Building Materials*, 53, 460–466. doi:10.1016/j.conbuildmat.2013.12.026

- Shalaby, A. (2002). Modelling short-term aging of asphalt binders using the rolling thin film oven test. *Canadian Journal of Civil Engineering*, 29(1), 135-144.
- Shen, S. (2006). *Dissipated energy concepts for HMA performance: Fatigue and healing*. ProQuest.
- Shen, S., & Carpenter, S. H. (2005). Application of the dissipated energy concept in fatigue endurance limit testing. *Transportation Research Record: Journal of the Transportation Research Board*, 1929(-1), 165-173.
- Shen, S., & Carpenter, S. H. (2007). Development of an asphalt fatigue model based on energy principles (with discussion and closure). *Journal of the Association of Asphalt Paving Technologists, 76*
- Shen, S., Chiu, H. M., & Huang, H. (2010). Characterization of fatigue and healing in asphalt binders. *Journal of Materials in Civil Engineering*, 22, 846.
- Sutharsan, T. (2010). Washington State University). Quantification of Cohesive Healing of Asphalt Binder Based on Dissipated Energy Analysis,
- Thodesen, C., Shatanawi, K., & Amirkhanian, S. (2009). Effect of crumb rubber characteristics on crumb rubber modified (CRM) binder viscosity. *Construction and Building Materials*, 23(1), 295-303.
- Tsai, B. W., Monismith, C., Dunning, M., Gibson, N., D'Angelo, J., Leahy, R., Davis, R. (2005). Influence of asphalt binder properties on the fatigue performance of asphalt concrete pavements. *Journal of the Association of Asphalt Paving Technologists*, 74, 733–789.
- Uddin, W. (2003). Viscoelastic characterization of polymer-modified asphalt binders of pavement applications. *Applied Rheology*, *13*(4), 191-199.
- Wahhab, H., Al-Dubabe, I., Asi, I., & Ali, M. (1998). Performance-based characterization of arab asphalt. *Building and Environment*, 33(6), 375-383.
- Wang, L., Chang, C., & Xing, Y. (2011). Creep characteristics of crumb rubber modified asphalt binder. *Journal of Wuhan University of Technology--Materials Science Edition*, 26(1), 118-120.
- Wu, S., Liu, G., Mo, L., Chen, Z., & Ye, Q. (2006). Effect of fiber types on relevant properties of porous asphalt. *Transactions of Nonferrous Metals Society of China*, 16, s791-s795.
- Wu, S., Pang, L., Liu, G., & Zhu, J. (2010). Laboratory study on ultraviolet radiation aging of bitumen. *Journal of Materials in Civil Engineering*, 22, 767.
- Wu, S., Ye, Q., & Li, N. (2008). Investigation of rheological and fatigue properties of asphalt mixtures containing polyester fibers. *Construction and Building Materials*, 22(10), 2111–2115.

- Wu, S., Ye, Q., Li, N., & Yue, H. (2007). Effects of fibers on the dynamic properties of asphalt mixtures. *Journal of Wuhan University of Technology--Materials Science Edition*, 22(4), 733-736.
- Xiao, F., Amirkhanian, S., & Juang, C. H. (2009). Prediction of fatigue life of rubberized asphalt concrete mixtures containing reclaimed asphalt pavement using artificial neural networks. *Journal of Materials in Civil Engineering*, 21, 253.
- Xiao, F., Wenbin Zhao, P., & Amirkhanian, S. N. (2009). Fatigue behavior of rubberized asphalt concrete mixtures containing warm asphalt additives. *Construction and Building Materials*, 23(10), 3144-3151.
- Yan, K., Xu, H., & Zhang, H. (2013). Effect of mineral filler on properties of warm asphalt mastic containing Sasobit. *Construction and Building Materials*, 48, 622–627. doi:10.1016/j.conbuildmat.2013.07.085
- Ye, Q., Wu, S., Chen, Z., & Liu, Z. (2008). Rheological properties of asphalt mixtures containing various fibers. *Journal of Central South University of Technology*, 15, 333-336.
- Ye, Q., Wu, S., & Li, N. (2009). Investigation of the dynamic and fatigue properties of fiber-modified asphalt mixtures. *International Journal of Fatigue*, *31*(10), 1598–1602.
- Zhang, J., Sabouri, M., Guddati, M. N., & Kim, Y. R. (2013). Development of a failure criterion for asphalt mixtures under fatigue loading. *Road Materials and Pavement Design*, 14(sup2), 1–15. doi:10.1080/14680629.2013.812843
- Zhang, W. (1997). Viscoelastic analysis of diametral compression of asphalt concrete. *Journal of Engineering Mechanics*, 123, 596.
- Zhou, F., Mogawer, W., & Li, H. (2012). Evaluation of Fatigue Tests for Characterizing Asphalt Binders. *Journal of Materials in ...*, 25(5), 610–617. doi:10.1061/(ASCE)MT.1943-5533.0000625.