



UNIVERSITI PUTRA MALAYSIA

***DEVELOPMENT OF RECTANGULAR RUBBER ISOLATORS FOR A
TUNNEL-FORM STRUCTURE SUBJECTED TO SEISMIC EXCITATIONS***

ALTALABANI DIYAR NASIH QADER

FK 2021 52



**DEVELOPMENT OF RECTANGULAR RUBBER ISOLATORS FOR A
TUNNEL-FORM STRUCTURE SUBJECTED TO SEISMIC EXCITATIONS**

By

ALTALABANI DIYAR NASIH QADER

**Thesis Submitted to the School of Graduate Studies, Universiti Putra
Malaysia, in Fulfilment of the Requirements for the Degree of Doctor of
Philosophy**

April 2021

All material contained within the thesis, including without limitation text, logos, icons, photographs and all other artwork, is copyright material of Universiti Putra Malaysia unless otherwise stated. Use may be made of any material contained within the thesis for non-commercial purposes from the copyright holder. Commercial use of material may only be made with the express, prior, written permission of Universiti Putra Malaysia.

Copyright © Universiti Putra Malaysia



Abstract of thesis presented to the Senate of Universiti Putra Malaysia in
fulfilment of the requirement for the degree of Doctor of Philosophy

DEVELOPMENT OF RECTANGULAR RUBBER ISOLATORS FOR A TUNNEL-FORM STRUCTURE SUBJECTED TO SEISMIC EXCITATIONS

By

ALTALABANI DIYAR NASIH QADER

April 2021

Chairman : Associate Professor Farzad Hejazi, PhD
Faculty : Engineering

The tunnel-form building system is gaining popularity due to its characteristics in time saving in construction. However, the tunnel-form structure is more vulnerable against lateral loads because of its large weight to stiffness ratio, which can potentially intensify the forces induced to the structure during seismic events. To improve this vulnerability during earthquake hazards, seismic base isolation systems can be incorporated into the tunnel-form structure. Among others, rubber isolators have been adopted extensively as effective base isolation systems for various building and bridge structures around the world. However, the most common shapes used for base isolators are circular and square, as the anisotropic isolation system has typically been designed by employing symmetrical-shaped seismic isolators. While, these configurations are not applicable for the tunnel-form building system, since the distribution of the loads corresponding to the shear wall does not provide a uniform support condition along the walls. Therefore, an attempt was made in this study to develop rectangular rubber isolators with couple cores that are applicable for tunnel-form structures. A rectangular isolator with dual lead cores instead of a single core is proposed to enhance the efficiency of the isolator along the direction of the wall in terms of lateral shear resistance and energy absorption capacity. Previous studies had revealed the poisoning effects of lead material exposure on the environment and human health. Thus, to avoid adverse effects caused by exposure to lead material, a rectangular isolator with dual rubber cores instead of lead cores was developed. In this study, the rubber cores in a rectangular isolator are confined with a single layer of CFRP wrap and stainless steel tube to improve the lateral shear behavior and damping ratio of the isolators. For the sake of comparison, a rectangular rubber isolator without cores is also considered in this study. Five full-scale rectangular isolators are manufactured and experimentally tested under a vertical compressive load and horizontal displacements to derive and evaluate their hysteresis response. Finite element models for the 5 mentioned large-

scale isolators are developed, and their performance under cyclic loads is investigated. The experimental and numerical results were then compared and they showed a good agreement. Based on the experimental testing and simulation results, the CFRP and stainless-steel tube confinement were found effective in improving the behavior of rectangular isolators with rubber cores in terms of damping ratio and energy dissipation capacity.

Furthermore, the proposed rectangular isolators are implemented into a 5-story tunnel-form structure, and nonlinear dynamic analyses were conducted for different structural performance levels. The seismic performance of the fixed base and base-isolated buildings is investigated by performing the Incremental Dynamic Analysis (IDA) using a suite of 10 pairs of earthquake ground motion records. Also, the fragility curves were created based upon the results of incremental dynamic analysis as it is one of the effective methods of conducting nonlinear dynamic analyses to gather data to estimate the fragility curves. In all models, the results showed that the probabilities of exceeding the Immediate Occupancy (I.O) performance level for coupling beams under both DBE and MCE hazard levels are less than 10 and 20%, respectively. This way, under both DBE and MCE hazard scenarios, these values for the walls are about 3 and less than 6%, respectively. It can be concluded that the tunnel-form structure can practically satisfy the Immediate Occupancy (I.O) performance level by implementing the proposed rectangular isolator systems even under severe seismic excitations.

Finally, the finite element parametric study based on the validated finite element models is conducted on 12 rectangular rubber isolators with one, two, and four square lead and rubber cores subjected to lateral cyclic loads. As in the case of prototype samples, the square rubber cores are confined with CFRP and steel layers which play a vital role, as concluded earlier, in improving the damping parameter of the proposed rectangular isolators. The numerical parametric study results showed a slight increase in the damping ratio with increasing the number of rubber cores. The results also indicated no remarkable difference between isolators tested along the length and those tested along the width. This shows that the response of the isolator does not depend on its shape rather than it is dependent on the amount and number of lead/rubber cores.

Abstrak tesis yang dikemukakan kepada Senat Universiti Putra Malaysia
sebagai memenuhi keperluan untuk ijazah Doktor Falsafah

PEMBANGUNAN ISOLATOR GETAH SEGI EMPAT TEPAT BAGI STRUKTUR BENTUK TEROWONG DI BAWAH PENGUJAJAN SEISMOS

Oleh

ALTALABANI DIYAR NASIH QADER

April 2021

Pengerusi : Profesor Madya Farzad Hejazi, PhD
Fakulti : Kejuruteraan

Sistem bangunan berbentuk terowong semakin popular kerana ciri-ciri penjimatan masa dalam pembinaan. Walau bagaimanapun, struktur terowong lebih rentan terhadap beban lateral kerana nisbah berat dan kekakuannya yang besar, yang berpotensi dapat meningkatkan kekuatan yang disebabkan struktur pada kejadian seismik. Untuk memperbaiki kerentanan ini semasa bahaya gempa, sistem pengasingan pangkalan seismik dapat dimasukkan ke dalam struktur terowong dari. Antara lain, isolator getah telah diadopsi secara meluas sebagai sistem pengasingan asas yang berkesan untuk pelbagai struktur bangunan dan jambatan di seluruh dunia. Walau bagaimanapun, bentuk yang paling umum digunakan untuk pengasingan asas adalah berbentuk bulat dan persegi, kerana sistem pengasingan anisotropik biasanya dirancang dengan menggunakan pengasingan seismik berbentuk simetris. Walaupun, konfigurasi ini tidak berlaku untuk sistem bangunan bentuk terowong, kerana pengedaran beban yang sesuai dengan dinding ricih tidak memberikan keadaan sokongan yang seragam di sepanjang dinding. Oleh itu, percubaan dibuat dalam kajian ini untuk mengembangkan isolator getah segi empat dengan pasangan inti yang dapat digunakan untuk struktur bentuk terowong. Isolator segi empat tepat dengan dua teras utama dan bukannya satu teras dicadangkan untuk meningkatkan kecekapan pengasingan sepanjang arah dinding dari segi ketahanan ricih lateral dan keupayaan penyerapan tenaga. Kajian terdahulu telah menunjukkan kesan keracunan pendedahan bahan plumbum terhadap persekitaran dan kesihatan manusia. Oleh itu, untuk mengelakkan kesan buruk yang disebabkan oleh pendedahan kepada bahan plumbum, sebuah isolator segi empat tepat dengan teras getah berganda dan bukannya teras plumbum dikembangkan. Dalam kajian ini, inti getah dalam isolator segi empat dibatasi dengan lapisan tunggal CFRP wrap dan tiub keluli tahan karat untuk memperbaiki tingkah laku ricih lateral dan nisbah redaman isolator. Sebagai perbandingan, pengasingan getah segi empat tanpa teras juga dipertimbangkan dalam kajian ini. Lima pengasingan segi empat tepat berskala

penuh dihasilkan dan diuji secara eksperimen di bawah beban mampatan menegak dan anjakan mendatar untuk mendapatkan dan menilai tindak balas histeresisnya. Model elemen hingga untuk 5 isolator berskala besar yang disebutkan telah dikembangkan, dan prestasi mereka di bawah beban siklik diselidiki. Hasil eksperimen dan berangka kemudian dibandingkan dan mereka menunjukkan persetujuan yang baik. Berdasarkan hasil ujian dan simulasi eksperimen, CFRP dan pengikat tiub tahan karat didapati berkesan dalam meningkatkan tingkah laku pengasing segi empat tepat dengan teras getah dari segi nisbah redaman dan kapasiti pelepasan tenaga.

Selanjutnya, isolator segi empat tepat yang dicadangkan diimplementasikan ke dalam struktur terowong 5 tingkat, dan analisis dinamik nonlinier dilakukan untuk tahap prestasi struktur yang berbeza. Prestasi seismik bangunan tetap dan terencil pangkalan disiasat dengan melakukan Analisis Dinamik Tambahan (IDA) menggunakan rangkaian 10 pasang rekod gerakan tanah gempa. Juga, lengkungan kerapuhan dibuat berdasarkan hasil analisis dinamik tambahan kerana ia adalah salah satu kaedah berkesan untuk melakukan analisis dinamik nonlinier untuk mengumpulkan data untuk menganggarkan keluk kerapuhan. Dalam semua model, hasilnya menunjukkan bahawa kebarangkalian melebihi tahap prestasi Penghunian Segera (I.O) untuk gandingan gandingan di bawah tahap bahaya DBE dan MCE masing-masing kurang dari 10 dan 20%. Dengan cara ini, di bawah senario bahaya DBE dan MCE, nilai-nilai untuk dinding masing-masing sekitar 3 dan kurang dari 6%. Dapat disimpulkan bahawa struktur bentuk terowong secara praktikal dapat memenuhi tahap prestasi Penghunian Segera (I.O) dengan menerapkan sistem isolator segi empat tepat yang dicadangkan walaupun dalam keadaan gegaran seismik yang teruk.

Akhirnya, kajian parametrik elemen hingga berdasarkan model elemen terhingga yang disahkan dilakukan pada 12 isolator getah segi empat dengan teras timah dan getah satu, dua, dan empat persegi yang dikenakan beban siklik lateral. Seperti dalam contoh prototaip, inti getah persegi dibatasi dengan lapisan CFRP dan keluli yang memainkan peranan penting, seperti yang disimpulkan sebelumnya, dalam meningkatkan parameter redaman isolator segi empat tepat yang dicadangkan. Hasil kajian parametrik berangka menunjukkan sedikit peningkatan nisbah redaman dengan peningkatan bilangan teras getah. Hasilnya juga menunjukkan tidak ada perbezaan yang luar biasa antara isolator yang diuji sepanjang dan yang diuji sepanjang lebarnya. Ini menunjukkan bahawa tindak balas pengasing tidak bergantung pada bentuknya daripada bergantung pada jumlah dan bilangan teras plumbum / getah.

ACKNOWLEDGEMENTS

I would like to express my deepest gratitude to my supervisor, Assoc. Prof. Dr. Farzad Hejazi for his dynamic guidance, caring, patience, and providing me with an excellent atmosphere for doing research. The blessing, help, and guidance given by him time to time shall carry me a long way in the journey of life on which I am about to embark. Without his unflinching encouragement, it would have been impossible for me to finish this research.

Besides my advisor, I would like to thank the rest of my thesis supervisory committee: Prof. Ir. Dr. Raizal Saifulnaz bin Muhammad Rashid and Assoc. Prof. Dr. Farah Nora Aznieta binti Abd Aziz for their insightful comments and encouragement. I offer my sincere appreciation for the learning opportunities provided by them.

My sincere thanks to the Hercules Engineering Company in collaboration with University Putra Malaysia (UPM) for fabricating and testing the rectangular rubber isolators used in this research work.

Last but not least; I would like to thank my family for supporting me spiritually throughout writing this thesis and my life in general.

This thesis was submitted to the Senate of Universiti Putra Malaysia and has been accepted as fulfilment of the requirement for the degree of Doctor of Philosophy. The members of the Supervisory Committee were as follows:

Farzad Hejazi, PhD

Associate Professor
Faculty of Engineering
Universiti Putra Malaysia
(Chairman)

Farah Nora Aznieta binti Abd Aziz, PhD

Associate Professor
Faculty of Engineering
Universiti Putra Malaysia
(Member)

Raizal Saifulnaz bin Muhammad Rashid, PhD

Professor Ir.
Faculty of Engineering
Universiti Putra Malaysia
(Member)

ZALILAH MOHD SHARIFF, PhD

Professor and Dean
School of Graduate Studies
Universiti Putra Malaysia

Date: 12 August 2021

TABLE OF CONTENTS

	Page
ABSTRACT	i
ABSTRAK	iii
ACKNOWLEDGEMENTS	v
APPROVAL	vi
DECLARATION	viii
LIST OF TABLES	xiii
LIST OF FIGURES	xv
LIST OF ABBREVIATIONS	xx
CHAPTER	
1 INTRODUCTION	1
1.1 Introduction	1
1.2 Research Hypothesis	3
1.3 Brief Review of Earlier Works	4
1.4 Problem Statement	6
1.5 Objectives of The Study	6
1.6 Scopes of The Study	7
1.7 Thesis Outlines	7
2 LITERATURE REVIEW	8
2.1 Introduction	8
2.2 The Seismicity of Malaysia	9
2.3 Industrialized Building System (IBS)	10
2.4 Performance of Tunnel-Form Buildings	13
2.5 Earthquake Resistant Buildings	19
2.5.1 Seismic Isolation Devices	20
2.5.1.1 Elastomeric Isolators	21
2.5.1.2 Lead Rubber Isolators	21
2.5.1.2.1 Health Effects of Lead Exposure	21
2.5.2 Behavior of Base Isolation Systems	22
2.6 Modeling Techniques	33
2.6.1 Nonlinear Finite Element Analysis	34
2.6.2 Modeling of Rubber Isolators	35
2.6.3 Verifications and Validations in Finite Element Analysis	38
2.7 Seismicity Design Requirements	40
2.7.1 Strength Requirements	40
2.7.2 Ductility Requirements	41
2.7.3 Stiffness Requirements	41
2.7.4 Equivalent Viscous Damping	42
2.8 Summary	42

3	MATERIALS AND METHODS	43
3.1	Introduction	43
3.2	Design of Rubber Isolators	45
3.3	Development of Proposed Rectangular Rubber Isolators	46
3.3.1	Geometrical Properties	48
3.3.2	Material Properties	52
3.3.2.1	Rubber Material	52
3.3.2.2	Lead Material	53
3.3.2.3	CFRP Material	54
3.3.2.4	Stainless Steel Material	55
3.4	Fabrication and Experimental Program	57
3.5	Numerical Modeling	61
3.5.1	Loading and Interaction	62
3.5.2	Material Modeling	62
3.5.2.1	Rubber Material	63
3.5.2.2	Lead Material	64
3.5.2.3	CFRP Material	64
3.5.2.4	Stainless Steel Material	64
3.6	Finite Element Parametric Study	65
3.6.1	Model Geometry	65
3.6.2	Loading and Interaction	66
3.6.3	Model Meshing	70
3.7	Application of The Proposed Isolators to The Tunnel-Form Structure	70
3.7.1	Nonlinear Modeling and Determination of Strength and Deformation Parameters	72
3.8	Summary	74
4	RESULTS AND DISCUSSION	75
4.1	Introduction	75
4.2	Experimental Results	75
4.3	Finite Element Modeling Results	78
4.4	Nonlinear Dynamic Analysis	81
4.5	Incremental Dynamic Analysis (IDA)	82
4.6	Seismic Reliability Analysis	90
4.7	Parametric Study Results	92
4.7.1	Rubber Isolators with Lead Cores	93
4.7.2	Rubber Isolators with Rubber Cores	94
4.7.3	Nonlinear Dynamic Analysis	96
4.7.4	Incremental Dynamic Analysis (IDA)	98
4.7.5	Seismic Reliability Analysis	102
4.8	Summary	104
5	CONCLUSIONS AND RECOMMENDATIONS	105
5.1	Conclusions	105
5.2	Recommendations	106
	REFERENCES	107
	APPENDIX A	118



LIST OF TABLES

Table		Page
2.1	Previous recorded earthquakes in Malaysia up to September 2007	9
2.2	Consideration of rectangular seismic isolators in shear wall buildings	33
3.1	Parameters for one-half of rectangular rubber isolators	51
3.2	Mechanical properties of rubber used in this study	52
3.3	Mechanical specifications of the lead material	54
3.4	Parameters of the Yeoh hyperelastic material used for the rubber	63
3.5	Hysteresis loop parameters of the rubber material	64
3.6	Model parameters of the parametric study models	68
3.7	The values of dead and live loads	71
3.8	Limit states for drifts (Δ) and chord rotation (θ) (rad)	73
4.1	Experimental results of rectangular rubber isolators	78
4.2	Performance of the rectangular rubber isolator models against experimental results	80
4.3	The natural vibration period (T) and the coefficient of translational effective mass factor (M) for a 5-story tunnel-form building	82
4.4	Selected ground motion records adopted in this study	83
4.5	The probabilities of reaching different performance levels (Immediate Occupancy I.O, Life Safety L.S, and Collapse Prevention C.P) in both DBE and MCE hazard levels (%)	92
4.6	Damping parameters for rectangular rubber isolators with varying lead cores and shear strain values	94

4.7	Damping parameters for rectangular rubber isolators under varying square rubber cores with and without CFRP/Steel confinement and 200% shear strain	96
4.8	The period of vibration (T) and the coefficient of translational effective mass factor (M) for a 5-story tunnel-form building	97
4.9	The probabilities of reaching different performance levels (Immediate Occupancy I.O, Life Safety L.S, and Collapse Prevention C.P) in both DBE and MCE hazard levels (%)	104



LIST OF FIGURES

Figure		Page
1.1	Sea seismic activities surrounding Malaysia	3
2.1	Interaction between three major tectonic plates around Malaysia	10
2.2	Construction process of tunnel-form building system	12
2.3	Tunnel-form building located in Izmit has no damage during the earthquake in 1999	14
2.4	Post-earthquake condition of the tunnel-form building in Izmit	15
2.5	Tunnel-form buildings after the Bingol earthquake in 2003	15
2.6	3D and plan views of 10-story tunnel-form building	18
2.7	3D and plan views of the irregular 10-story tunnel-form building	19
2.8	Seismic isolation systems	20
2.9	Common base isolator types	20
2.10	View of used lead rubber isolators in all columns	23
2.11	Schematic representation of (a) 3D rubber isolator with lead core and its components, (b) side view, and (c) plan view	24
2.12	Dimensions and element selection	25
2.13	Meshing details of elastomeric isolator in ABAQUS: (a) side view and boundary conditions of bottom loading plate, (b) 3D view, and (c) section view	25
2.14	Schematic view of the lead rubber damper	26
2.15	Cutaway view of a base-isolated nuclear power plant	27

2.16	Modeling of a lead rubber isolator	27
2.17	Overall setup of tested highway bridge model assembled on the shake table	28
2.18	Experimental setup of full-scale rubber isolators	29
2.19	Experimental setup of reduced-scale rubber isolators	29
2.20	3D schematic view of the proposed isolator and its components	30
2.21	Rubber isolators with rubber cores	30
2.22	Components of proposed Multi-Directional Seismic Isolation (MDSI) system	31
2.23	Schematic view of the fabrication sequence of a seismic isolator	32
2.24	The idealized hysteresis behavior of a bilinear mechanical model of all isolators used in this study	36
2.25	Summary flowchart for the preliminary design of rubber isolators	38
2.26	Verification and validation processes	39
3.1	General flowchart of the research methodology	44
3.2	Schematic representation of the isolation system of (a) masonry structure with rectangular strip isolators, (b) masonry structure with circular/square isolators, and (c) frame structure with square isolators	47
3.3	Schematic view of the fabrication procedure of a rectangular rubber isolator with two rubber cores confined with a single layer of CFRP sheet and stainless steel tube	49
3.4	Components of a large-size rectangular rubber isolator with two lead/rubber cores	50
3.5	Four unconfined cylindrical rubber cores	52
3.6	Tensile testing machine of rubber dumbbell	53

3.7	Stress-strain results of rubber dumbbells	53
3.8	Four cylindrical lead cores	54
3.9	Four cylindrical rubber cores (a) before and (b) after confinement with CFRP sheet	55
3.10	Four rubber cores confined with stainless steel tube	55
3.11	Typical tensile test on steel coupon sample	56
3.12	Stress-strain relationships for stainless steel tube material	56
3.13	Failure mode of coupon samples	57
3.14	Rectangular rubber isolators used in this study	58
3.15	Overall view of the testing machine and programmable logic controller system	59
3.16	Rectangular rubber isolators tested along their length and width	60
3.17	The model of a rectangular rubber isolator and its deformed shape with mapped mesh	61
3.18	Loading steel plates of a rectangular rubber isolator with reference points	62
3.19	Schematic view of the fabrication procedure of a rectangular rubber isolator with two square rubber cores confined with a single layer of CFRP/steel (parametric study)	66
3.20	Loading steel plate of a rectangular isolator with a reference point (parametric study)	67
3.21	The model of rectangular rubber isolator and its deformed shape with mapped mesh (parametric study)	70
3.22	Plan and 3D views of the 5-story tunnel-form building	71
3.23	Reinforcement detailing of walls and coupling beams	72

3.24	The behavior of nonlinear shear-control element of walls and coupling beams (a) adopted in the performance-based design software, and (b) prescribed by ASCE41-13	73
3.25	The schematic illustration of the selected deformation-controlled parameters for (a) story drift (Δ) in shear walls, and (b) chord rotation (θ) for coupling beams as proposed by ASCE41-13 when shear dominates the inelastic response	73
4.1	Experimental results of hysteresis diagrams of rectangular rubber isolators tested along both orthogonal directions	78
4.2	Comparison of the numerical analysis and experimental hysteresis diagrams of rectangular rubber isolators tested along both orthogonal directions	80
4.3	Acceleration response spectrum of the selected ground motions scaled to their PGA with 5% damping	84
4.4	Comparison of the mean values of the maximum displacement of mass centers between fixed base and various isolation systems for DBE and MCE levels	85
4.5	Comparison of the mean values of the maximum shear forces between fixed base and various isolation systems for DBE and MCE levels	86
4.6	Position of the early damage initiation in the walls and coupling beams in the case of MCE hazard scenario	87
4.7	Results of the Incremental Dynamic Analysis (IDA) and limit states for 5-story tunnel-form building structure with various isolation systems	90
4.8	Fragility curves for 5-story tunnel-form building structure with various isolation systems	91
4.9	Results of shear force-horizontal displacement hysteresis analysis of rubber isolators with varying square lead cores tested along (a) length and (b) width (parametric study)	93

4.10	Hysteretic shear force-horizontal displacement curves of rubber isolators with a single central square rubber core with and without CFRP/Steel confinement tested along (a) length and (b) width (parametric study)	95
4.11	Hysteretic shear force-horizontal displacement curves of rubber isolators with two square rubber cores with and without CFRP/Steel confinement tested along (a) length and (b) width (parametric study)	95
4.12	Hysteretic shear force-horizontal displacement curves of rubber isolators with four square rubber cores with and without CFRP/Steel confinement tested along (a) length and (b) width (parametric study)	96
4.13	Distribution of rectangular isolators under the walls	97
4.14	Comparison of the mean values of the maximum displacement of mass centers between fixed base and various isolation systems for DBE and MCE levels (parametric study)	98
4.15	Comparison of the mean values of the maximum shear forces between fixed base and various isolation systems for DBE and MCE levels (parametric study)	99
4.16	Position of the early damage initiation in the walls and coupling beams in the case of MCE hazard scenario (parametric study)	100
4.17	Results of the Incremental Dynamic Analysis (IDA) and limit states for 5-story tunnel-form building structure with various isolation systems (parametric study)	102
4.18	Fragility curves for 5-story tunnel-form building structure with various isolation systems (parametric study)	103

LIST OF ABBREVIATIONS

A	Cross-Sectional Area
ACI	American Concrete Institute
a_g	Ground Acceleration
A_i	Area of Isolator
A_l	Area of Lead Core
A_{lead}	Cross-Sectional Area of Lead Core
A_r	Area of Rubber Core
A_{rubber}	Cross-Sectional Area of Rubber Core
$ASCE$	American Society of Civil Engineers
$ASTM$	American Society for Testing and Materials
ATC	Applied Technology Council
$ATC3-06$	Amended Tentative Provisions for The Development of Seismic Regulations for Buildings
$BS8110$	British Design Code
$C.P$	Collapse Prevention
$CFRP$	Carbon Fiber Reinforced Polymer
$CIDB$	Construction Industry Development Board
$CIMP$	Construction Industry Master Plan
D	Lateral Displacement
$D.L$	Dead Load
DBE	Design Basis Earthquake
D_i	Diameter of Isolator
D_{lead}	Diameter of Lead Core

DM	Damage Measure
DOF	Degree of Freedom
D_{rubber}	Diameter of Rubber Core
DS	Damage State
D_y	Yield Displacement
E	Elastic Modulus of Steel
E_c	Rubber Compression Modulus
$EC8$	Eurocode Design
EI	Bending Stiffness
FEA	Finite Element Analysis
FEM	Finite Element Modeling
$FEMA$	Federal Emergency Management Agency
F_y	Yield Strength of Lead/Rubber Material
G	Rubber Shear Modulus
h	Total Height of Isolator
H_{lead}	Height of Lead Core
H_{rubber}	Height of Rubber Core
H_w	Height of Shear Wall Segment
I	Importance Factor
$I.O$	Immediate Occupancy
I_1	First Invariant of The Strain Deviator
IBS	Industrialized Building System
IDA	Incremental Dynamic Analysis
IDR	Interstory Drift Ratio

<i>IM</i>	Intensity Measure
<i>IRHD</i>	International Rubber Hardness Degrees
<i>J_{el}</i>	Volume of Elastic Strain
<i>K</i>	Stiffness
<i>k</i>	Rubber Bulk Modulus
<i>K₁</i>	Elastic Stiffness
<i>K₂</i>	Post-Yield Stiffness
<i>K_{eff}</i>	Effective Stiffness
<i>K_H</i>	Horizontal Stiffness
<i>K_{h,eff}</i>	Horizontal Effective Stiffness
<i>KLIA</i>	Kuala Lumpur International Airport
<i>K_r</i>	Total Rubber Stiffness
<i>K_v</i>	Vertical Stiffness
<i>L.L</i>	Live Load
<i>L.S</i>	Life Safety
<i>L_l</i>	Length of Loading Steel Plate
<i>L_r</i>	Length of Rubber Pad
<i>L_s</i>	Length of Sealing Steel Plate
<i>LS</i>	Limit State
<i>L_{sh}</i>	Length of Steel Shim Plate
<i>L_w</i>	Length of Shear Wall Segment
<i>M</i>	Coefficient of Translational Effective Mass Factor
<i>MCE</i>	Maximum Considered Earthquake
<i>MGDM</i>	Mineral and Geosciences Department of Malaysia

<i>MGDM</i>	Mineral and Geosciences Department of Malaysia
<i>MMI</i>	Modified Mercalli Intensity
N_{lead}	Number of Lead Cores
<i>NPP</i>	Nuclear Power Plant
n_r	Number of Rubber Layers
N_{rubber}	Number of Rubber Cores
n_{sh}	Number of Steel Shim Layers
<i>NZ310</i>	New Zealand Design Code
<i>PEER</i>	Pacific Earthquake Engineering Research Center
<i>PGA</i>	Peak Ground Acceleration
<i>PLC</i>	Programmable Logic Controller
<i>Q</i>	Characteristic Strength
Q_G	Gravity Load
<i>R</i>	Distance to The Closest Fault Rupture
<i>RC</i>	Reinforced Concrete
R_i	Lateral Force Coefficient
<i>RP</i>	Reference Point
<i>S</i>	Shape Factor
<i>SDOF</i>	Single Degree of Freedom
<i>SR</i>	Slip Rate
<i>SSI</i>	Soil-Structure Interaction
<i>T</i>	Effective Period
t_{CFRP}	Thickness of CFRP Sheet
T_D	Design Basis Earthquake (DBE) Time Period

t_l	Single Loading Steel Plate Thickness
T_M	Maximum Considered Earthquake (MCE) Time Period
T_r	Total Rubber Thickness
t_r	Single Rubber Thickness
t_s	Single Sealing Steel Plate Thickness
t_{sh}	Single Steel Shim Thickness
t_{steel}	Thickness of Stainless Steel Layer
<i>UBC97</i>	Uniform Building Code 97
<i>USGS</i>	United States Geological Survey
W	Density of The Strain Energy
W_d	Area of Hysteresis Loop
<i>WHO</i>	World Health Organization
W_l	Width of Loading Steel Plate
W_r	Width of Rubber Pad
W_s	Width of Sealing Steel Plate
W_s	Restored Energy
W_{sh}	Width of Steel Shim Plate
Y	Realized Condition of Ground Motion
Z	Seismic Zone Factor
β_{eff}	Effective Damping
γ	Rubber Shear Strain
Δ	Drift
ϵ_{Eng}	Engineering Strain of Steel
ϵ_{true}	True Strain of Steel

ζ_{eq}	Equivalent Viscous Damping
θ	Chord Rotation
μ	Ductility
ν	Poisson's Ratio
σ_{Eng}	Engineering Stress of Steel
σ_{true}	True Stress of Steel
Φ	Diameter of Rubber Pad
Φ_D	Diagonal Steel Reinforcement
Φ_H	Horizontal Steel Reinforcement
Φ_{HT}	Horizontal Transverse Steel Reinforcement
Φ_L	Longitudinal Steel Reinforcement
Φ_V	Vertical Steel Reinforcement
Φ_{VT}	Vertical Transverse Steel Reinforcement
ω	Natural Frequency
ξ	Damping Ratio

CHAPTER 1

INTRODUCTION

1.1 Introduction

In most living areas, an earthquake as a devastating event affects its habitants, so the primary objective of earthquake scientists and researchers was to minimize the earthquake irreversible damage. The use of nonlinear dynamic analyses was expanded by passing the time and switching points of view from strength-based plotting to performance-based plotting. The performance-based design is the modern approach that was commonly used in the latest instructions and regulations (ASCE 41-13, 2013; ATC-40, 1996; FEMA 273, 1997; FEMA 350, 2000; FEMA 356, 2000; FEMA P695, 2009). During seismic excitations, some tensions are generated in structures that advance the structural components toward yield and collapse (Providakis, 2008). The first stage in the seismic improvement of existing structures is the seismic vulnerability analysis. The vulnerabilities and deficiencies of structures are defined based on the evaluation methods carried out on them. Primarily, several limitations are existed in selecting the enhancement methods of structures that impact the adopted approach. One of the most economical and efficient strategies is to separate the foundation from the superstructure to prevent the damage caused by the earthquake, which decreases the energy generated by the earthquake without affecting the structure's stability. The seismic performance-based design is the modern design approach used for evaluating the performance of structures through conducting the Incremental Dynamic Analysis (IDA) which includes scaling ground motion records to cover the entire domain of structure response, from elasticity to yielding, and then global dynamic instability (Mansouri et al., 2017).

In high seismicity areas, seismic base isolator systems are frequently used to ensure the seismic protection of essential facilities. Earthquake experts have developed and installed various seismic base isolation systems (Kim et al., 2019). Among others, seismic rubber isolators have been adopted extensively as isolation systems for various building and bridge structures worldwide. Although it is not possible to control the earthquake itself, its actual impact on the building can be controlled by mitigating the effects of the foundation's movement on the superstructure by employing base isolators. Through installing seismic isolators between the foundation and superstructure, the isolation system offers improved stability and energy dissipation capacity. Although the use of conventional seismic isolator systems may be familiar in various countries around the world, rectangular rubber isolators have rarely been investigated for shear wall structures (Al-Kutti & Islam, 2019; Ismail et al., 2010; Islam & Al-Kutti, 2018; Ounis & Ounis, 2013; Saha et al., 2015). The design of base-isolated structures is primarily based on the assumption that

when subjected to seismic excitations, the structure will effectively respond elastically. It is predicted that the damage caused by seismic forces will be concentrated in the isolators that are designed to be easily replaceable. As their design is aligned with the concepts of resilience, i.e., rapid restoration and low-damage, isolated structures are resilient. The use of steel shim layers alternating with rubber layers provides improved vertical stiffness to preserve the structure's self-weight and the isolator's lateral flexibility (Kalfas et al., 2017).

The Industrialized Building System (IBS) is the construction process that most of its structural components such as walls, slabs, beams, columns, and staircases are pre-prepared in the factory and assembled on the site in a short amount of time with high-quality construction. This system is effective in projects that provide repetitive formation opportunities with today's refinements. Because of its industrialized modular construction method, the more repetitive steps there are, the greater the benefits. The system is especially suitable for multi-unit housing, student housing, military housing, hotels, single-family homes, townhouses, and prisons (Qasem, 2016). The tunnel-form building system is classified under the IBS which consists of shear walls and flat slabs. There are two primary functions of shear walls: carrying vertical loads (load-bearing walls) and resisting lateral loads such as wind and seismic loads. During the construction process of the tunnel-form structural system, cast-in-situ concrete is poured into two inverted L-shape steel formworks to simultaneously build the shear walls and flat slabs in a daily cycle. For every 24 hours, the formwork is transferred to another building level. Upon finishing each story level, the construction process is repeated on the next floor and as a result, the residential units can be built up quickly. For this purpose, the tunnel-form system is a unique alternative to repetitive plans for erecting medium to high-rise buildings. In this innovative system, precast components don't exist in which the structural members such as shear walls and slabs are poured in place having almost the same thickness. This leads to minimize the number of joints and result in a symmetrical configuration in vertical and horizontal plan views of a monolithic structure, as well as to provide high seismic resistance.

Malaysia is located close to the most seismically active region known as the earthquakes zone and volcanic eruption encircling the Pacific Ring of Fire. However, Malaysia is deemed a low seismic zone under a far-field earthquake from Sumatra and near-field earthquakes from Bukit Tinggi to Kuala Lumpur fault lines (Hamid et al., 2014). The majority of existing reinforced concrete (RC) buildings in Malaysia have been designed based on the British BS8110 code to sustain gravity loads, notional lateral loads, and wind loads, which has no specific provision for seismic loads (Qasem, 2016). Nevertheless, due to the regularly felt ground tremors from earthquake events in the neighboring countries and earthquakes of local origins that occurred in Malaysia, RC buildings may be at risk under a repetitive low cycle of failure that leads to significant damage of these buildings. For instance, the reinforced concrete school building in Sabah has experienced partial damages under an earthquake with a magnitude of 4.3 scale Richter. These damages indicate that

a structure's overall performance could not sustain under a low magnitude of seismic excitation. If an unexpected earthquake occurs within 300 km of the epicenter of Malaysia, a significant collapse of these structures can happen (Hamid et al., 2014). Figure 1.1 shows the seismic activities surrounding the east and west of Malaysia (Syahrum, 2007).

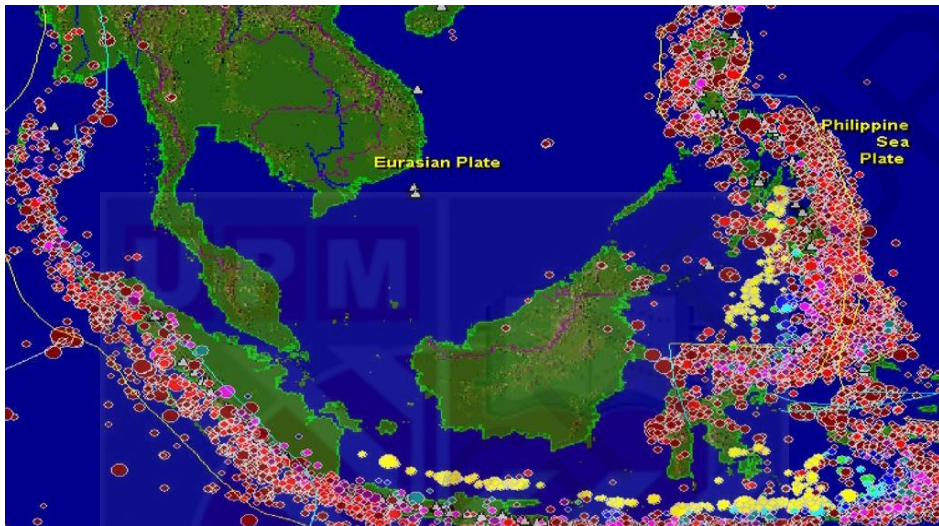


Figure 1.1: Sea seismic activities surrounding Malaysia
(Source: Syahrum, 2007)

1.2 Research Hypothesis

Energy dissipation covers any component used to reduce the movement of structures under dynamic excitation (e.g., wind, earthquakes). From the literature review, it is further observed that the majority of conventional lead rubber bearing isolators available in the market have a single lead core which is not sufficient for shear wall structures due to its great weight and stiffness. Therefore, increasing the number and amount of lead cores is crucial to achieving the high damping performance of the proposed rectangular rubber isolators along the wall direction of the tunnel-form building system. Although there are limited studies on rectangular isolators for shear wall structures, proper experimental tests and physical and material modeling of rectangular rubber isolators are not seen. On the other hand, the previous research studies proved the poisoning effects of lead material exposure on the environment and human health. Therefore, to avoid any harmful effects caused by the lead material, a rectangular isolator with rubber cores instead of lead cores will be developed. The literature review revealed that the rubber cores in the conventional base isolators are unconfined which resulted in a low viscous damping ratio and unable to withstand the great weight and stiffness of tunnel-form structures under seismic activity and vibration. Thus, the rubber cores will

be confined with a single layer of CFRP wrap and stainless steel tube to increase the damping ratio of the isolation system via dissipating more energy and vibrations induced to the building. This study aims to develop a new type of seismic base isolator that uses a combination of lead or rubber cores to improve energy dissipation. Full-scale experimental testing will be conducted. A numerical model will be developed to investigate the device considering lateral cyclic loading. Scenarios involving different types of plugs will be considered; the stiffness and damping of the devices will be discussed. In the second half of the study, nonlinear time history analysis will be performed on a base-isolated tunnel-form building with the bearings mentioned above. The structure's response will be evaluated based on base shear, drift, and damage of the concrete structural elements. An incremental dynamic analysis will be performed, and fragility curves will be generated.

1.3 Brief Review of Earlier Works

Seismic rubber isolators are common types of seismic isolation systems. Numerous studies have been investigated under seismic excitation for mechanical simulation of these isolators. Cho et al. (2016) conducted comparative research on various models of rubber hyperelastic materials. The analysis is performed on lead rubber isolators with different rubber material properties. The study revealed that the Ogden model captures the material with fewer errors than other hyperelastic models. Ahmadipour and Alam (2017) presented the mechanical properties of a square lead rubber isolator with a sensitivity analysis. The research compared the isolators' horizontal and vertical stiffness to various lead types, lead radius, and rubber layers. Their findings revealed that the radius of the lead core was the most influential parameter among the three variables above in effecting the quality of the lead rubber isolator. The degradation of vertical stiffness of laminated rubber isolators was performed under lateral shear loading by Yang et al. (2017). The analysis results revealed that the ratio of vertical stiffness is calculated only by the section proportion of the lateral deformation to the inertia radius, and is not affected by the section shape, direction of loading (compressive or tensile), and isolator size. Kalfas et al. (2017) and Kalfas and Mitoulis (2017) used finite element methods to examine the response of laminated rubber isolators under the effect of different axial loads. The study results indicated that increasing axial compressive load caused a remarkable decrease in the isolator stiffness. Their results indicated that the isolator demonstrates a fluctuate behavior when considering the combination of shear strain, axial loading, and rotation. Zeynali et al. (2018) carried out an experimental and numerical investigation on the use of lead rubber dampers in the chevron braced frame. They concluded that the isolator with a larger diameter of the lead core has a greater capacity for energy dissipation. Kumar and Whittaker (2018) proposed a mathematical model capable of providing the best estimate of the isolator's response under extreme load. Xiang et al. (2018) conducted a shake table test of a highway bridge by enabling laminated rubber isolators to be slid with and without restraining equipment. Gauron et al. (2018) used experimental results to test the shear failure and lateral stability limit states of rubber bridge isolators. The

study findings demonstrated that the limited operating conditions seem to be virtually impossible, and why the majority of isolators do not show any damage before failure cannot be determined experimentally. Rahnavard and Thomas (2019) performed a theoretical study on the mechanical properties of elastomeric base isolators with various rubber cores under combined tensile and compressive loading effects. Their study showed that rubber isolators with different radially distributed circular rubber cores overcome those with a single central rubber core. In a more recent study, Rahnavard et al. (2020) performed a numerical investigation on the static and dynamic lateral stability of circular and square rubber isolators with different rubber cores. They concluded that using single and multiple rubber cores increases the lateral stability of the isolators due to the high critical axial forces.

The innovative construction industry is gaining popularity in the structural engineering community and progressing increasingly towards more effective structural systems and technologies to minimize expenses, construction time, and human resources, as well as to improve the durability and stability of buildings during severe loading excitations such as intense earthquakes. However, the novel tunnel-form building systems may deliver many advantages in this regard, such as professional planning capacity, shortening the construction time, and ultimately contributing to a rapid return on assets (Mohsenian et al., 2019). All the vertical load-bearing members of this unique system are composed of walls and slabs classified as lateral and vertical load-bearing components, which are cast in each story simultaneously. Since the walls and slabs are built simultaneously in each level, using a cold joint to maintain an integrated performance of the structure throughout an earthquake event is unnecessary (Tavafoghi & Eshghi, 2008). Although such structural systems are extensively being used in densely populated urban areas and industrial projects, relatively few studies in this field have been performed. There is absolutely no specification or standard that describes this novel system as a different load-bearing structural system. An analysis of the code-based relationships for determining the fundamental vibration period found that the values obtained from the empirical relationships were either notably underestimated or overestimated; this inconsistency between the actual and empirical results demonstrated a deficiency in predicting the seismic demands (Mohsenian et al., 2019).

Balkaya and Kalkan (2003) and Balkaya and Kalkan (2004) suggested some mathematical relationships to measure the fundamental vibration periods of 2 and 5-story tunnel-form building structures with a varying number of stories and plans through performing a variety of pushover analyses. Correspondingly, the 3D membrane behavior was observed to be a dominant force function in such buildings. Finally, for both shorter and taller buildings, response modification factors equal to 5 and 4 have been suggested. Yuksel and Kalkan (2007) and Kalkan and Yuksel (2008) examined the 3D behavior of intersection walls by conducting several tests on samples containing minimum reinforcement percentage for the small longitudinal reinforcement ratio. Using computer-aided design software, the analysis results showed that increasing longitudinal

reinforcement bars in the corners of the walls would have a desirable effect on their response and could vary the brittle fracture failure even in small reinforcement ratios. Tavafoghi and Eshghi (2008) performed different studies on tunnel-form building structures and their findings revealed that the fundamental period of vibration has depended on the building's height in both directions. Thus, the number of walls, as well as the aspect ratio, were having a negligible impact on the building's fundamental vibration period. Besides, Aval et al. (2018) assessed the seismic performance of the tunnel-form building that was subject to both near-field and far-field ground motion records using the effects of forward directivity. Based on the results, it has been indicated that forward directivity can affect the modes of failure of a tall tunnel-form building structure and minimize the design reliability. Recently, Mohsenian et al. (2019) evaluated the seismic performance of tunnel-form building structures, considering the soil-structure interaction effect. Depending on their numerical findings, the effects of SSI on the structural response such as story displacement, story shear force, and position of damage initiation became more significant with an increase in building heights and earthquake intensities.

1.4 Problem Statement

The problem statement of this study which derived through reviewing of the literature is summarized as follow:

- 1- Currently, the most common isolators are available in a circular or square shape which is not applicable for the tunnel-form buildings as isolation systems, since the distribution of the shear wall loads (gravity loads) does not create a uniform support condition along the walls.
- 2- The majority of conventional isolators have a single lead core which is insufficient for a tunnel-form structure due to its great weight and stiffness. Therefore, increasing the number and amount of lead core is essential to enhance the efficiency of the rectangular isolators along the wall direction of the tunnel-form system.
- 3- Pervious research studies and international organizations are suggested to eliminate lead material due to its harmful effects on the environment and human health. Thus, an alternative material instead of the lead material is required to implement.
- 4- The literature revealed that the rubber cores in the seismic isolators are unconfined, which resulted in a low energy absorption capacity. Therefore, confining the rubber cores with CFRP/steel layers is crucial since it acts as a full bounded jacketing to provide a remarkable lateral shear capacity and damping ratio for the rectangular isolators.

1.5 Objectives of The Study

The primary objectives of this research work can be summarized as follow:

- 1- To increase the viscous damping ratio of the rectangular rubber isolators for a tunnel-form structure subjected to seismic events.
- 2- To experimentally investigate five full-scale rectangular rubber isolators with double lead and rubber cores under combined vertical and horizontal loads.
- 3- To numerically propose rectangular rubber isolators with different square lead and rubber cores based on the validated finite element models.
- 4- To apply the developed rectangular rubber isolators into 5-story base-isolated tunnel-form structures and compare the results with the seismic performance of the fixed base structure.

1.6 Scopes of The Study

The primary scopes of this study are summarized as follow:

- 1- The energy dissipation capacity of the rectangular isolators was increased by increasing the amount of the lead cores and confining the rubber cores with a CFRP sheet and stainless steel tube.
- 2- The proposed design for the rectangular rubber isolators was evaluated through finite element simulations and experimental tests under combined vertical compressive and shear loadings.
- 3- The finite element parametric study was conducted based on the validated models in order to propose rectangular isolators with a variety of square lead and rubber cores.
- 4- The Incremental Dynamic Analysis (IDA) was performed and the fragility curves were generated in order to assess the seismic performance of the base-isolated buildings.

1.7 Thesis Outlines

Chapter One presents the introduction and general philosophy of the study. The problem statement and objectives of the present study are also illustrated in this chapter. **Chapter Two** reviews the previous research works related to the base isolation devices and their applications in the buildings. The seismic performance of the tunnel-form structures, including earlier researches in this field, is also presented. The experimental and numerical procedures of the proposed rectangular isolators are described in **Chapter Three** along with a brief explanation of the material properties and specifications used in this study. Experimental and numerical results are summarized and discussed in **Chapter Four**. The finite element parametric study based on the validated models is also conducted and its results are examined in this chapter. **Chapter Five** draws attention to embody the study's conclusions and additional recommendations intended for future research works.

REFERENCES

- Abaqus. (2014). ABAQUS user's manual. Version 6.14.
- ACI Committee, & International Organization for Standardization. (2019). Building code requirements for structural concrete (ACI 318-19) and commentary. American Concrete Institute.
- Agrawal, P., & Shrikhande, M. (2006). Earthquake resistant design of structures. PHI Learning Pvt. Ltd.
- Ahmadipour, M., & Alam, M. S. (2017). Sensitivity analysis on mechanical characteristics of lead-core steel-reinforced elastomeric bearings under cyclic loading. *Engineering Structures*, 140, 39-50.
- Akopyan, K., Petrosyan, V., Grigoryan, R., & Melkorian, D. M. (2018). Assessment of residential soil contamination with arsenic and lead in mining and smelting towns of northern Armenia. *Journal of Geochemical Exploration*, 184, 97-109.
- Al-Aghbari, A., Hamid, N., Rahman, N., & Hamzah, S. (2012). Structural performance of two types of wall slab connection under out-of-plane lateral cyclic loading. *Journal of Engineering science and technology*, 7(2), 177-194.
- Al-Kutti, W. A., & Islam, A. B. M. (2019). Potential design of seismic vulnerable buildings incorporating lead rubber bearing. *Buildings*, 9(2), 37.
- American Society for Testing and Materials. ASTM. (2018). Designation: D 412 – 06a Standard test method for tensile properties of Vulcanized Rubber and Thermoplastic Elastomers—. ASTM D 412-06a, 598, 143–152.
- American Society of Civil Engineers (2013). *Seismic Rehabilitation of Existing Buildings* (ASCE/SEI 41-13).
- Andriyana, A., Loo, M. S., Chagnon, G., Verron, E., & Ch'ng, S. Y. (2015). Modeling the Mullins effect in elastomers swollen by palm biodiesel. *International Journal of Engineering Science*, 95, 1-22.
- Ang, A. H. S., & Tang, W. H. (2007). *Probability concepts in engineering planning and design: Emphasis on application to civil and environmental engineering*. Wiley.
- Anuar, S. A., & Hamid, N. H. A. (2014). Comparison of seismic performance between single and double unit tunnel form building under in-plane lateral cyclic loading. In *Applied mechanics and materials* (Vol. 567, pp. 687-692). Trans Tech Publications Ltd.

- Applied Technology Council (1996). Seismic Evaluation and Retrofit of Concrete Buildings (ATC-40).
- Applied Technology Council (2009). Quantification of Building Seismic Performance Factors (ATC-63).
- ASCE (American Society of Civil Engineers). (2016). Minimum design loads and associated criteria for buildings and other structures. ASCE standard ASCE/SEI 7-16 (in preparation). Reston, VA: American Society of Civil Engineers.
- Aval, S. B. B., Mohsenian, V., & Kouhestani, H. S. (2018). Seismic performance-based assessment of tunnel form building subjected to near-and far-fault ground motions. *Asian Journal of Civil Engineering*, 19(1), 79-92.
- Baker, J. W. (2015). Efficient analytical fragility function fitting using dynamic structural analysis. *Earthquake Spectra*, 31(1), 579-599.
- Balkaya, C., & Kalkan, E. (2003). Estimation of fundamental periods of shear-wall dominant building structures. *Earthquake engineering & structural dynamics*, 32(7), 985-998.
- Balkaya, C., & Kalkan, E. (2003). Seismic design parameters for shear-wall dominant building structures. The 14th national congress on earthquake engineering, guanajuato, Mexico, 19-22.
- Balkaya, C., & Kalkan, E. (2004). Seismic vulnerability, behavior and design of tunnel form building structures. *Engineering Structures*, 26(14), 2081-2099.
- Balkaya, C., & Kalkan, E. (2004, August). Relevance of R-factor and fundamental period for seismic design of tunnel-form building. In 13th World Conference on Earthquake Engineering, Vancouver, Canada.
- Berahman, F., & Behnamfar, F. (2007). Seismic fragility curves for un-anchored on-grade steel storage tanks: Bayesian approach. *Journal of Earthquake Engineering*, 11(2), 166-192.
- Bergström, J. S., & Boyce, M. C. (1998). Constitutive modeling of the large strain time-dependent behavior of elastomers. *Journal of the Mechanics and Physics of Solids*, 46(5), 931-954.
- Bose-O'Reilly, S., Yabe, J., Makumba, J., Schutzmeier, P., Ericson, B., & Caravanos, J. (2018). Lead intoxicated children in Kabwe, Zambia. *Environmental research*, 165, 420-424. <https://doi.org/10.1016/j.envres.2017.10.024>

- Carrington, C., Devleeschauwer, B., Gibb, H. J., & Bolger, P. M. (2019). Global burden of intellectual disability resulting from dietary exposure to lead, 2015. *Environmental research*, 172, 420-429.
- Cho, S. G., Park, W. K., & Yun, S. M. (2016). Finite element analysis of lead rubber bearing by using strain energy function of hyper-elastic material. *Journal of The Korean Society of Civil Engineers*, 36(3), 361-374.
- CIDB (2003b). IBS Survey 2003. Malaysia: Construction Industry Development Board.
- CIDB (2011). Proceedings of 6th IBS Roundtable Workshop: Experiences and Lesson Learnt on Industrialized Building System (IBS) Construction in Malaysia. Malaysia, CREAM: Construction Research Institute of Malaysia.
- Cimellaro, G. P., & Marasco, S. (2018). Introduction to Dynamics of Structures and Earthquake Engineering (Vol. 45). Springer.
- Computers and Structures Inc. (CSI). (2018). Structural and earthquake engineering software, PERFORM-3D nonlinear analysis and performance assessment for 3-D structures. Version 6.0. 0.
- Computers and Structures Inc. CSI (2016). Structural and earthquake engineering software, ETABS, extended three-dimensional analysis of building systems nonlinear version 16.2.1, berkeley, CA, USA.
- De Luca, A., & Guidi, L. G. (2020). Base isolation issues in Italy: Integrated architectural and structural designs. *Soil Dynamics and Earthquake Engineering*, 130(November 2019), 105912.
- EN 1337-3 (2005). Structural bearings Part 3: elastomeric bearings. Brussels: European-Committee for Standardization.
- Ersoy, U. (1988). Seismic Resistant Reinforced Concrete Structures-Design Principles. *Medical Journal of Islamic World Academy of Sciences*, 1(1), 20-26.
- Federal Emergency Management Agency (1997). Guidelines for Seismic Rehabilitation of Buildings (FEMA 273).
- Federal Emergency Management Agency (2000). Prestandard and Commentary for the Seismic Rehabilitation of Buildings (FEMA 350).
- Federal Emergency Management Agency (2000). Recommended Seismic Design Criteria for New Steel Moment-Frame Buildings (FEMA 350).
- Federal Emergency Management Agency (2009). Quantification of Building System Performance and Response Parameters (FEMA P695).

- Federal Emergency Management Agency (2009). Quantification of building seismic performance factors (FEMA P695, ATC-63). FEMA P695, prepared by the Applied Technology Council; 421p.
- Federal Emergency Management Agency. (2003). Recommended provisions for seismic regulation for new buildings and other structures (FEMA 450-1).
- Garcia, L. E., & Sozen, M. A. (2004). Earthquake resistant design of reinforced concrete buildings. From Engineering Seismology to Performance-Based Engineering.
- Gauron, O., Saidou, A., Busson, A., Siqueira, G. H., & Paultre, P. (2018). Experimental determination of the lateral stability and shear failure limit states of bridge rubber bearings. *Engineering Structures*, 174, 39-48.
- Habieb, A. B., Valente, M., & Milani, G. (2019). Base seismic isolation of a historical masonry church using fiber reinforced elastomeric isolators. *Soil Dynamics and Earthquake Engineering*, 120, 127-145.
- Habieb, A. B., Valente, M., & Milani, G. (2019). Effectiveness of different base isolation systems for seismic protection: Numerical insights into an existing masonry bell tower. *Soil Dynamics and Earthquake Engineering*, 125, 105752.
- Hadian, A. V., Mutalib, A. A., & Baharom, S. (2013). Seismic behaviour of base isolation system using lead rubber bearing. *Jurnal Teknologi*, 65(2).
- Hajirasouliha, I., Pilakoutas, K., & Mohammadi, R. K. (2016). Effects of uncertainties on seismic behaviour of optimum designed braced steel frames. *Steel and Composite Structures*, 20(2), 317-335.
- Hamid (2011). Seismic Behavior of Shear-Key Precast Wall Panel in Double-Story House Subjected to Lateral Cyclic Loading. Seminar Teknikal Gempabumi 20-21 December 2011 Jabatan Meteorology Malaysia, Petaling Jaya, Selangor.
- Hamid, N. H., Saleh, S. M., & Anuar, S. A. (2014). Seismic performance of double-unit tunnel form building under in-plane lateral cyclic loading. *Structures Under Shock and Impact XIII*, 141, 467.
- Hassanein, M. F. (2010). Numerical modelling of concrete-filled lean duplex slender stainless steel tubular stub columns. *Journal of constructional steel research*, 66(8-9), 1057-1068.
- Hassanein, M. F., Kharoob, O. F., & Liang, Q. Q. (2013). Behaviour of circular concrete-filled lean duplex stainless steel-carbon steel tubular short columns. *Engineering structures*, 56, 83-94.

- Hassim, S., Jaafar, M. S., & Sazalli, S. A. (2009). The contractor perception towers industrialised building system risk in construction projects in Malaysia. *American Journal of applied sciences*, 6(5), 937.
- Islam, A. B. M., & Al-Kutti, W. A. (2018). Seismic response variation of multistory base-isolated buildings applying lead rubber bearings. *Computers and Concrete*, 21(5), 495-504.
- Islam, A. S., Jameel, M., & Jumaat, M. Z. (2011). Seismic isolation in buildings to be a practical reality: Behaviour of structure and installation technique. *Journal of Engineering and Technology Research*, 3(4), 99-117.
- Ismail, M., Rodellar, J., & Ikhouane, F. (2010). An innovative isolation device for aseismic design. *Engineering Structures*, 32(4), 1168-1183.
- Jerrams, S. J., Kaya, M., & Soon, K. F. (1998). The effects of strain rate and hardness on the material constants of nitrile rubbers. *Materials & design*, 19(4), 157-167.
- Kalfas, K. N., & Mitoulis, S. A. (2017). Performance of steel-laminated rubber bearings subjected to combinations of axial loads and shear strains. *Procedia engineering*, 199, 2979-2984.
- Kalfas, K. N., Mitoulis, S. A., & Katakalos, K. (2017). Numerical study on the response of steel-laminated elastomeric bearings subjected to variable axial loads and development of local tensile stresses. *Engineering Structures*, 134, 346-357.
- Kalkan, E., & Yüksel, S. B. (2008). Pros and cons of multistory RC tunnel-form (box-type) buildings. *The Structural Design of Tall and Special Buildings*, 17(3), 601-617.
- Kelly, J. M. (1999). Analysis of fiber-reinforced elastomeric isolators. *Journal of Seismology and Earthquake Engineering*, 2(1), 19.
- Kelly, J. M. (2002). Seismic isolation systems for developing countries. *Earthquake Spectra*, 18(3), 385-406.
- Kelly, J. M., & Takhirov, S. M. (2002). Analytical and experimental study of fiber-reinforced strip isolators. PEER Report 2002/11, Pacific Earthquake Engineering Research Center. University of California, Berkeley.
- Kim, J. H., Kim, M. K., & Choi, I. K. (2019). Experimental study on seismic behavior of lead-rubber bearing considering bi-directional horizontal input motions. *Engineering Structures*, 198, 109529.
- Klasanovic, I., Kraus, I., & Hadzima-Nyarko, M. (2014). Dynamic properties of multistory reinforced concrete tunnel-form building-a case study in

- Osijek, Croatia. Forecast Engineering: Global Climate change and the challenge for built environment, 17-29.
- Kumar, A., Singh, B. P., Kumar, K., Kumar, M., & Kumar, M. (2017). A Study on Earthquake Resistant. 5811–5818.
- Kumar, M., & Whittaker, A. S. (2018). Cross-platform implementation, verification and validation of advanced mathematical models of elastomeric seismic isolation bearings. *Engineering Structures*, 175, 926-943.
- Kumar, M., & Whittaker, A. S. (2018). Cross-platform implementation, verification and validation of advanced mathematical models of elastomeric seismic isolation bearings. *Engineering Structures*, 175, 926-943.
- Kumar, M., Whittaker, A. S., & Constantinou, M. C. (2014). An advanced numerical model of elastomeric seismic isolation bearings. *Earthquake engineering & structural dynamics*, 43(13), 1955-1974.
- Kumar, M., Whittaker, A. S., & Constantinou, M. C. (2015). Seismic isolation of nuclear power plants using sliding bearings. Technical Rep. MCEER-15-0006. New York: Univ. at Buffalo.
- Lead smelting; http://www.worstpolluted.org/projects_reports/display/86
- Lee, C. M., Terrizzi, A. R., Bozzini, C., Piñeiro, A. E., Conti, M. I., & Martínez, M. P. (2016). Chronic lead poisoning magnifies bone detrimental effects in an ovariectomized rat model of postmenopausal osteoporosis. *Experimental and Toxicologic Pathology*, 68(1), 47-53.
- Logan, D. L. (2011). *A first course in the finite element method*. Cengage Learning.
- Malaysian Meteorological Department, (2012). *Seismicity in Malaysia and around the Region*.
- Mansouri, I., Ghodrati Amiri, G., Hu, J. W., Khoshkalam, M., Soori, S., & Shahbazi, S. (2017). Seismic fragility estimates of LRB base isolated frames using performance-based design. *Shock and Vibration*, 2017.
- Matsagar, V. A., & Jangid, R. S. (2004). Influence of isolator characteristics on the response of base-isolated structures. *Engineering structures*, 26(12), 1735-1749.
- MGDM, (2009). *Mineral and Geosciences Department of Malaysia*.
- Mohsenian, V., & Nikkhoo, A. (2019). A study on the effects of vertical mass irregularity on seismic performance of tunnel-form structural system. *Advances in concrete construction*, 7(3), 131-141.

- Mohsenian, V., Nikkhoo, A., & Hajirasouliha, I. (2019). Estimation of seismic response parameters and capacity of irregular tunnel-form buildings. *Bulletin of Earthquake Engineering*, 17(9), 5217-5239.
- Mohsenian, V., Nikkhoo, A., & Hejazi, F. (2019). An investigation into the effect of soil-foundation interaction on the seismic performance of tunnel-form buildings. *Soil Dynamics and Earthquake Engineering*, 125, 105747.
- Muntasir Billah, A. H. M., & Shahria Alam, M. (2015). Seismic fragility assessment of highway bridges: a state-of-the-art review. *Structure and Infrastructure Engineering*, 11(6), 804-832.
- Naeim, F., & Kelly, J. M. (1999). *Design of seismic isolated structures: from theory to practice*. John Wiley & Sons.
- Nazri, F. M. (2011). *Predicting the collapse potential of structures in earthquakes* (Doctoral dissertation, University of Bristol).
- Nazri, F. M., & Curves, F. F. (2018). *Seismic fragility assessment for buildings due to earthquake excitation*. Singapore: Springer.
- Nguyen, Q. T., Tinard, V., & Fond, C. (2015). The modelling of nonlinear rheological behaviour and Mullin's effect in High Damping Rubber. *International Journal of Solids and Structures*, 75, 235-246.
- Ohsaki, M., Miyamura, T., Kohiyama, M., Yamashita, T., Yamamoto, M., & Nakamura, N. (2015). Finite-element analysis of laminated rubber bearing of building frame under seismic excitation. *Earthquake Engineering & Structural Dynamics*, 44(11), 1881-1898.
- Osgoeei, P. M., Tait, M. J., & Konstantinidis, D. (2016). Seismic isolation of a shear wall structure using rectangular fiber-reinforced elastomeric isolators. *Journal of Structural Engineering*, 142(2), 04015116.
- Osgoeei, P. M., Van Engelen, N. C., Konstantinidis, D., & Tait, M. J. (2015). Experimental and finite element study on the lateral response of modified rectangular fiber-reinforced elastomeric isolators (MR-FREIs). *Engineering Structures*, 85, 293-303.
- Ounis, H. M., & Ounis, A. (2013). Parameters influencing the response of a base-isolated building. *Slovak Journal of Civil Engineering*, 21(3), 31-42.
- Pan, T. C., & Sun, J. (1996). Historical earthquakes felt in Singapore. *Bulletin of the Seismological Society of America*, 86(4), 1173-1178.
- Patel, V. I., Hassanein, M. F., Thai, H. T., Al Abadi, H., & Paton-Cole, V. (2017). Behaviour of axially loaded circular concrete-filled bimetallic stainless-carbon steel tubular short columns. *Engineering Structures*, 147, 583-597.

- Pourmasoud, M. M., Lim, J. B., Hajirasouliha, I., & McCrum, D. (2020). Multi-Directional Base Isolation System for Coupled Horizontal and Vertical Seismic Excitations. *Journal of Earthquake Engineering*, 1-26.
- Providakis, C. P. (2008). Effect of LRB isolators and supplemental viscous dampers on seismic isolated buildings under near-fault excitations. *Engineering structures*, 30(5), 1187-1198.
- Qasem, M. M. A. (2016). Seismic performance comparison between 3-story of tunnel form building and (RC) frame system using 3D non-linear finite element analysis [Unpublished master's thesis]. University Putra Malaysia.
- Rahnavard, R., & Thomas, R. J. (2019). Numerical evaluation of steel-rubber isolator with single and multiple rubber cores. *Engineering Structures*, 198, 109532.
- Rahnavard, R., Craveiro, H. D., & Napolitano, R. (2020, August). Static and dynamic stability analysis of a steel-rubber isolator with rubber cores. In *Structures* (Vol. 26, pp. 441-455). Elsevier.
- Rehman, Z. U., Khan, S., Brusseau, M. L., & Shah, M. T. (2017). Lead and cadmium contamination and exposure risk assessment via consumption of vegetables grown in agricultural soils of five-selected regions of Pakistan. *Chemosphere*, 168, 1589-1596.
- Rohan, (2017). <https://www.rohanbuilders.com/blog/tunnel-form-construction-technique-at-rohan-abhilasha-pune.html>
- Saedniya, M., & Talaeitaba, S. B. (2019). Numerical modeling of elastomeric seismic isolators for determining force–displacement curve from cyclic loading. *International Journal of Advanced Structural Engineering*, 11(3), 361-376.
- Saha, S. K., Matsagar, V. A., & Jain, A. K. (2015). Reviewing dynamic analysis of base-isolated cylindrical liquid storage tanks under near-fault earthquakes. *The IES Journal Part A: Civil & Structural Engineering*, 8(1), 41-61.
- Shahzad, M., Kamran, A., Siddiqui, M. Z., & Farhan, M. (2015). Mechanical characterization and FE modelling of a hyperelastic material. *Materials Research*, 18(5), 918-924.
- Shamilah et al (2015). Strengthening of Single Bay Tunnel Form Building Using Steel Angle, Steel Plate and CFRP Under Out-of-Plane Lateral Cyclic Loading. *Proceedings of 2015 International Conference on Disaster Management in Civil Engineering*, Phuket, Thailand, pp.21-26.
- Shamilah et al (2015). Strengthening of Single Bay Tunnel Form Building Using Steel Angle, Steel Plate and CFRP Under In-Plane Lateral Cyclic

Loading. Proceedings of 2015 International Conference on Disaster Management in Civil Engineering, Phuket, Thailand, pp.21-26.

- Sharif, A. M., Al-Mekhlafi, G. M., & Al-Osta, M. A. (2019). Structural performance of CFRP-strengthened concrete-filled stainless steel tubular short columns. *Engineering Structures*, 183, 94-109.
- Sheet, P. D. (2017). SikaWrap®-231 C, "WOVEN UNIDIRECTIONAL CARBON FIBRE FABRIC, DESIGNED FOR STRUCTURAL STRENGTHENING" Applications as part of the Sika strengthening system , Version 01.01, May . November, 2–5.
- Sheet, P. D. (2019). Sikadur®-330. October, 2–5.
- Shome, N. (1999). Probabilistic seismic demand analysis of nonlinear structures. Reliability of Marine Structures Report No: RMS-35, Civil and Environmental Engineering, Stanford University.
- Singh, N., & Li, J. H. (2014). Environmental impacts of lead ore mining and smelting. In *Advanced Materials Research* (Vol. 878, pp. 338-347). Trans Tech Publications Ltd.
- Skyscraper City, (2008). Tunnel Form Buildings in Turkey.
- Srinivas, K., Scientific, A., & Characterization, M. (2020). Verifications and Validations in Finite Element Analysis (FEA) Verifications and Validations in Finite Element Analysis (FEA) by Kartik Srinivas Advanced Scientific and Engineering Services (AdvanSES) (An Independent Material Testing Laboratory) 212 , Shukan Mall , Sabarmati-Gandhinagar Highway. May.
- Standard, ASTM. (2013). E8/E8M-13a. Standard Test Methods for Tension Testing of Metallic Materials, ASTM International, West Conshohocken, PA.
- Syahrum, S. (2007). Seismic Analysis and Design of Residential Building Based on Indonesian Code (Doctoral dissertation, Universiti Putra Malaysia).
- Talaeitaba, S. B., Pourmasoud, M. M., & Jabbari, M. (2019). An innovative base isolator with steel rings and a rubber core. *Asian Journal of Civil Engineering*, 20(3), 313-325.
- Tavafoghi, A., & Eshghi, S. (2008, October). Seismic behavior of tunnel form concrete building structures. In *The 14th World Conference on Earthquake Engineering* (pp. 12-17).
- Tiong, P. L. Y. (2014). Performance of load-bearing precast concrete wall with base isolation subjected to seismic loadings (Doctoral dissertation, Universiti Teknologi Malaysia).

- Tiong, P. L., Adnan, A., & Hamid, N. H. (2013). Behaviour factor and displacement estimation of low-ductility precast wall system under seismic actions. *Earthquakes and Structures*, 5(6), 625-655.
- Toopchi-Nezhad, H., Tait, M. J., & Drysdale, R. G. (2008). Testing and modeling of square carbon fiber-reinforced elastomeric seismic isolators. *Structural Control and Health Monitoring: The Official Journal of the International Association for Structural Control and Monitoring and of the European Association for the Control of Structures*, 15(6), 876-900.
- UBC, (1997, April). Uniform building code. In International Conference of Building Officials, Whittier, CA.
- Vamvatsikos, D., & Cornell, C. A. (2002). Incremental dynamic analysis. *Earthquake engineering & structural dynamics*, 31(3), 491-514.
- Vamvatsikos, D., & Cornell, C. A. (2004). Applied incremental dynamic analysis. *Earthquake Spectra*, 20(2), 523-553.
- Van Engelen, N. C., Osgoee, P. M., Tait, M. J., & Konstantinidis, D. (2014). Experimental and finite element study on the compression properties of Modified Rectangular Fiber-Reinforced Elastomeric Isolators (MR-FREIs). *Engineering Structures*, 74, 52-64.
- WHO, (2010). Exposure to Lead: A Major Public Health Concern.
- Xiang, N., Alam, M. S., & Li, J. (2018). Shake table studies of a highway bridge model by allowing the sliding of laminated-rubber bearings with and without restraining devices. *Engineering Structures*, 171, 583-601.
- Yáñez, L., García-Nieto, E., Rojas, E., Carrizales, L., Mejía, J., Calderón, J., ... & Díaz-Barriga, F. (2003). DNA damage in blood cells from children exposed to arsenic and lead in a mining area. *Environmental Research*, 93(3), 231-240.
- Yang, W., Sun, X., Wang, M., & Liu, P. (2017). Vertical stiffness degradation of laminated rubber bearings under lateral deformation. *Construction and Building Materials*, 152, 310-318.
- Yuksel, S. B., & Kalkan, E. (2007). Behavior of tunnel form buildings under quasi-static cyclic lateral loading. *Structural Engineering and Mechanics*, 27(1), 99.
- Zaini, S., Sawada, S., & Goto, H. (2012). Proposal for Seismic Resistant Design in Malaysia: Assessment of Possible Ground Motions in Peninsular Malaysia.
- Zeynali, K., Monir, H. S., Mirzai, N. M., & Hu, J. W. (2018). Experimental and numerical investigation of lead-rubber dampers in chevron

concentrically braced frames. Archives of Civil and Mechanical Engineering, 18, 162-178.

Zhang, X., Yang, L., Li, Y., Li, H., Wang, W., & Ye, B. (2012). Impacts of lead/zinc mining and smelting on the environment and human health in China. Environmental monitoring and assessment, 184(4), 2261-2273.



BIODATA OF STUDENT

Diyar was born in Kirkuk, Iraq on July 14th 1990. He received his degree in Bachelor of Science in July 2012 from College of Engineering, Civil Engineering Department, University of Kirkuk, Iraq. His Master degree in Structural Engineering was obtained from University of Mosul, Iraq in February 2015. From 2015 until 2018, he worked as an Assistant Lecturer in the Department of Civil and Architectural Engineering at Cihan University-Erbil, Iraq. He started preparing for the PhD degree in Structural Engineering at University Putra Malaysia (UPM) since July 2018 and completed it in August 2021. His primary research interests are in the field of Structural Engineering and Building Materials. Specifically, he is interested in Reinforced Concrete Structures, Finite Element Analysis, Structural Dynamics, Carbon Fiber Reinforced Polymer (CFRP), and Steel Structures. He can be contacted at diyarnasih90@gmail.com.

LIST OF PUBLICATIONS

- Altalabani, D., Hejazi, F., Rashid, R. S. B. M., & Abd Aziz, F. N. A. (2021). Development of new rectangular rubber isolators for a tunnel-form structure subjected to seismic excitations. *Structures*, 32, 1522-1542. (JCR Q2, I.F = 1.839)
- Altalabani, D., & Hejazi, F. (2021). Dynamic Response of a Tunnel-Form Structure Subjected to Seismic Excitations. *Design Engineering*, 1372-1385. (SCOPUS Q4, SJR = 0.1)





UNIVERSITI PUTRA MALAYSIA

STATUS CONFIRMATION FOR THESIS / PROJECT REPORT AND COPYRIGHT

ACADEMIC SESSION : _____

TITLE OF THESIS / PROJECT REPORT :

NAME OF STUDENT : _____

I acknowledge that the copyright and other intellectual property in the thesis/project report belonged to Universiti Putra Malaysia and I agree to allow this thesis/project report to be placed at the library under the following terms:

1. This thesis/project report is the property of Universiti Putra Malaysia.
2. The library of Universiti Putra Malaysia has the right to make copies for educational purposes only.
3. The library of Universiti Putra Malaysia is allowed to make copies of this thesis for academic exchange.

I declare that this thesis is classified as :

*Please tick (v)

CONFIDENTIAL

(Contain confidential information under Official Secret Act 1972).

RESTRICTED

(Contains restricted information as specified by the organization/institution where research was done).

OPEN ACCESS

I agree that my thesis/project report to be published as hard copy or online open access.

This thesis is submitted for :

PATENT

Embargo from _____ until _____
(date) (date)

Approved by:

(Signature of Student)
New IC No/ Passport No.:

Date :

(Signature of Chairman of Supervisory Committee)
Name:

Date :

[Note : If the thesis is CONFIDENTIAL or RESTRICTED, please attach with the letter from the organization/institution with period and reasons for confidentially or restricted.]