



UNIVERSITI PUTRA MALAYSIA

***BUCKLING AND POST-BUCKLING OF STEEL BOX SECTION USING
FINITE ELEMENT METHOD***

NUR HAZWANI BINTI ISHAM

FK 2021 81



**BUCKLING AND POST-BUCKLING OF STEEL BOX SECTION USING
FINITE ELEMENT METHOD**

By

NUR HAZWANI BINTI ISHAM

**Thesis Submitted to the School of Graduate Studies, Universiti Putra Malaysia,
in Fulfilment of the Requirements for the Degree of Master of Science**

January 2021

COPYRIGHT

All material contained within the thesis, including without limitation text, logos, icons, photographs, and all other artwork, is copyright material of Universiti Putra Malaysia unless otherwise stated. Use may be made of any material contained within the thesis for non-commercial purposes from the copyright holder. Commercial use of material may only be made with the express, prior, written permission of Universiti Putra Malaysia.

Copyright © Universiti Putra Malaysia



Abstract of thesis presented to the Senate of Universiti Putra Malaysia in fulfilment of the requirement for the degree of Master of Science

BUCKLING AND POST-BUCKLING OF STEEL BOX SECTION USING FINITE ELEMENT METHOD

By

NUR HAZWANI BINTI ISHAM

January 2021

Chairman : Noorfaizal bin Yidris, PhD
Faculty : Engineering

Typically components of structural member use combination of plate structures (i.e. plate elements) in many engineering applications. Many of these structures are subjected to in-plane compressive loading. During compression, the load increases while the stiffness maintained until it reaches the critical load whereby the out-of-plane deflections start to visible. As the load increases, the stiffness reduces to half of its pre-buckling stiffness and the deflections growth in stable manner. This is accompanied by continues alterations in the stress distributions within the cross section. It is therefore important to study the buckling and post-buckling behavior of such components. Most of the studies of square box-section struts in compression have been focusing on constant uniform thickness. This is due to the sections manufactured, available in the market and the standard used by the manufactures to form the sections. All square and rectangular sections have been formed with wall thickness uniform around the section. Nevertheless, it is possible to form the square box-sections with different thickness ratios. This gives more flexibility in design and reduce wastages. The research work sets out an investigation on behavior of the stress distributions of the plates and box section in the post-buckling region with regards to varying thickness ratios of the box section which are 1mm and 2mm. It was observed that the stress variations of box section with thickness ratio 1:1 and plate with thickness 1mm are similar while the stress variations of box section with thickness ratio 1:2 and plate with thickness 2mm are significantly different. A detailed account of the growth and redistributions of stresses across the section was given in the research work. The results from the finite element simulations were shown to compare well with the analytical method of analysis. The percentage difference of the numerical value for square plate model in this study was less than 10% when comparing with the theoretical value.

Abstrak thesis yang dikemukakan kepada Senat Universiti Putra Malaysia sebagai memenuhi keperluan untuk ijazah Master Sains

**LEDINGAN DAN PASCA-LEDINGAN KE ATAS KOMPONEN BESI
BERBENTUK SEGI EMPAT SAMA MENGGUNAKAN KAEDAH UNSUR
TERHINGGA**

Oleh

NUR HAZWANI BINTI ISHAM

Januari 2021

Pengerusi : Noorfaizal bin Yidris, PhD
Fakulti : Kejuruteraan

Biasanya komponen struktur anggota menggunakan kombinasi struktur plat (iaitu elemen plat) dalam banyak aplikasi kejuruteraan. Sebilangan besar struktur ini dikenakan beban pemampatan sesatah. Semasa pemampatan, beban meningkat sementara kekakuan dikekalkan sehingga mencapai satu beban kritikal di mana anjakan arah luar satah mulai kelihatan. Apabila beban meningkat, kekakuan berkurang sehingga separuh daripada kekakuan pra-ledingan dan anjakan luar satah bertambah dengan stabil. Ini disertai oleh perubahan berterusan dalam agihan tegasan dalam keratan rentas. Oleh itu, penting untuk mengkaji tingkah laku lekungan dan pasca-ledingan komponen tersebut. Sebilangan besar kajian mengenai strut berbentuk kotak dikenakan daya mampatan dengan memfokuskan pada ketebalan yang seragam. Ini disebabkan oleh pengilangan strut berbentuk kotak yang tersedia di pasaran dan standard yang digunakan oleh pengilang untuk membentuk bentuk kotak tersebut. Semua kotak bersegi empat sama dan bersegi empat tepat telah dibentuk dengan ketebalan dinding yang seragam di sekitar bahagian tersebut. Walaupun begitu, adalah mungkin untuk membentuk bentuk kotak dengan nisbah ketebalan yang berbeza. Ini memberikan lebih banyak pilihan dalam reka bentuk dan mengurangkan pembaziran. Kajian penyelidikan mengkaji tingkah laku taburan tekanan pada bahagian plat dan kotak di kawasan pasca-ledingan berkenaan dengan nisbah ketebalan yang berbeza iaitu 1mm dan 2mm. Telah diperhatikan bahawa variasi tegasan bahagian kotak dengan nisbah ketebalan 1:1 dan plat dengan ketebalan 1mm adalah serupa sementara variasi tegasan bahagian kotak dengan nisbah ketebalan 1:2 dan plat dengan ketebalan 2mm mempunyai perbezaan yang ketara. Laporan terperinci mengenai pertumbuhan dan pengagihan semula tekanan di seluruh bahagian diberikan dalam kajian penyelidikan. Hasil dari simulasi unsur terhingga menunjukkan perbandingan yang baik dengan simulasi bebas. Peratusan perbezaan bagi nilai berangka untuk model plat segi empat dalam kajian ini adalah kurang dari 10% apabila dibandingkan dengan nilai teori.

ACKNOWLEDGEMENTS

Alhamdulillah, applause to Allah s.w.t for giving me the discernment, toughness, willpower and determination, mostly because of His willingness I achieved success in having completed this Master's research project with entitled Buckling and Post-buckling of Steel Box Section.

First of all, I would really like to voice my sincere thanks to Dr. Noorfaizal Yidris, my key supervisor, for his continuous assistance, reassurance, inspiration, guidance and highly valued advocate through this whole study. The whole work would be a negligent endeavor without him participation.

In relation, I would also want to applaud my co-supervisor Dr Ezanee Gires for his guidance, impetus and encouragement on my research study. Moreover, I am forever indebted to my parents whose blessings are upon me. I think in my academic life, they took me far enough. Special mention to my inspiring father, Isham bin Hamzah and my beloved mother Norhaszalena binti Hashim. Not to forget, huge thanks to all my family and friends for their prayers, moral and financial support, encouragement and involuntary support throughout the hardest time finishing this research study. My driving force has been your help and your interests. For the meantime, I would like to thank all those who contributed directly and indirectly throughout this study by offering their ideas, motivations and support.

Finally, deepest thanks and appreciation to Universiti Putra Malaysia for taking me as one of Graduate Research Fellowship (GRF) for the last two semesters that was actually a big contribution to my life during this research study.

This thesis was submitted to the Senate of Universiti Putra Malaysia and has been accepted as fulfillment of the requirement for the degree of Master of Science. The members of the Supervisory Committee were as follows:

Noorfaizal bin Dato Yidris, PhD

Senior Lecturer
Faculty of Engineering
Universiti Putra Malaysia
(Chairman)

Ezanee bin Gires, PhD

Senior Lecturer
Faculty of Engineering
Universiti Putra Malaysia
(Member)

ZALILAH MOHD SHARIFF, PhD

Professor and Dean
School of Graduate Studies
Universiti Putra Malaysia

Date: 09 September 2021

Declaration by graduate student

I hereby confirm that:

- this thesis is my original work;
- quotations, illustrations and citations have been duly referenced;
- this thesis has not been submitted previously or concurrently for any other degree at any institutions;
- intellectual property from the thesis and copyright of thesis are fully-owned by Universiti Putra Malaysia, as according to the Universiti Putra Malaysia (Research) Rules 2012;
- written permission must be obtained from supervisor and the office of Deputy Vice-Chancellor (Research and innovation) before thesis is published (in the form of written, printed or in electronic form) including books, journals, modules, proceedings, popular writings, seminar papers, manuscripts, posters, reports, lecture notes, learning modules or any other materials as stated in the Universiti Putra Malaysia (Research) Rules 2012;
- there is no plagiarism or data falsification/fabrication in the thesis, and scholarly integrity is upheld as according to the Universiti Putra Malaysia (Graduate Studies) Rules 2003 (Revision 2012-2013) and the Universiti Putra Malaysia (Research) Rules 2012. The thesis has undergone plagiarism detection software

Signature: _____

Date: _____

Name and Matric No: Nur Hazwani binti Isham, GS53880

Declaration by Members of Supervisory Committee

This is to confirm that:

- the research conducted and the writing of this thesis was under our supervision;
- supervision responsibilities as stated in the Universiti Putra Malaysia (Graduate Studies) Rules 2003 (Revision 2012-2013) were adhered to.

Signature: _____

Name of Chairman
of Supervisory
Committee:

Dr. Noorfaizal bin Dato Yidris

Signature: _____

Name of Member
of Supervisory
Committee:

Dr. Ezanee bin Gires

TABLE OF CONTENTS

	Page
ABSTRACT	i
ABSTRAK	ii
ACKNOWLEDGEMENTS	iii
APPROVAL	iv
DECLARATION	vi
LIST OF TABLES	x
LIST OF FIGURES	xi
LIST OF ABBREVIATIONS	xiii
CHAPTER	
1 INTRODUCTION	1
1.1 Research Background	1
1.2 Problem Statement of the Research	2
1.3 Objectives of the Research	3
1.4 Scopes of the Research	3
1.5 Thesis Outline	4
2 LITERATURE REVIEW	6
2.1 Steel Structures	6
2.2 Finite Element Method (FEM)	8
2.3 Buckling and Post-Buckling Behavior of Thin Plate Structures	10
2.4 Buckling and Post-Buckling of Steel Box-section	15
2.5 The Thickness of Thin Plate in Box-section	18
2.6 Literature Review Summary	23
3 METHODOLOGY	24
3.1 Finite Element Analysis (FEA)	24
3.2 Geometry and Model of the Analysis	27
3.2.1 Part Module of the Model	29
3.3 Characterization of the Material	31
3.4 Boundary Condition and Load Applied	31
3.5 Verification of the Square Plate Model	34
3.5.1 Mesh Convergence Analysis of Square Plate	34
3.6 Validation of the Square Plate Model	35
3.6.1 Validation Study of Square Plate	35
4 RESULT AND DISCUSSION	39
4.1 Mode Shapes of Square Plates	39
4.2 Mode Shapes of Box-sections	40
4.3 Comparison of Post-buckling Behavior between Plates and Box-sections	42
4.3.1 Stress-Strain Behavior of Plates and Box-sections	42

4.3.2	Stress Distributions of Box A and Plate (t= 1mm)	44
4.3.3	Stress Distributions of Box B, Plate (t=1mm) and Plate (t=2mm)	46
4.4	Summary of the Results	51
5	CONCLUSIONS AND RECOMMENDATIONS	52
5.1	Conclusions	52
5.2	Recommendations for Future Research	53
	REFERENCES	54
	BIODATA OF STUDENT	60
	PUBLICATION	61



LIST OF TABLES

Table		Page
3.1	Geometry dimensions of the models used	28
3.2	Properties of steel used for the models	31
3.3	Boundary condition applied on the plate ($t=1\text{mm}$ and $t=2\text{mm}$) at all edges	32
3.4	Boundary condition applied on the Box A and B at all edges	33
3.5	The slope of the graph of non-dimensional load-end shortening variation for all the research work	37
3.6	Percentage difference between the FEM and previous research work	37

LIST OF FIGURES

Figure		Page
2.1	Load-displacement results for experimental and numerical method	9
2.2	Buckling behavior of perfect plate and imperfect plate	11
2.3	Post-buckling model of a thin plate under in-plane loads	13
2.4	Geometric properties of the rectangular hollow section strut	18
2.5	Test section	19
2.6	Cross-section geometry for welded and cold-formed columns	20
2.7	Cross-section and material properties of rectangular hollow section struts	20
2.8	Geometry of box strut cross-section	21
2.9	Matrix of stub column tests	22
2.10	Geometrical properties of main steel members	22
3.1	Flow chart of methodology	24
3.2	Flow chart of the steps for buckling and post-buckling of plates and boxes using ABAQUS	26
3.3	Square plate ($t=1\text{mm}$)	27
3.4	Square box A	27
3.5	Square plate ($t=2\text{mm}$)	28
3.6	Square box B	28
3.7	Geometry of plate ($t=1\text{mm}$) and ($t=2\text{mm}$) in ABAQUS software	29
3.8	Geometry of box-section thickness ratio 1:1 and 1:2 in ABAQUS software	29
3.9	Part constructed using shell features	30
3.10	The base feature	30
3.11	Boundary condition applied on the plate ($t=1\text{mm}$) and ($t=2\text{mm}$)	32

3.12	Boundary condition applied on the box A and B	32
3.13	Boundary condition applied at side A on both Box A and B	33
3.14	Boundary condition applied at side B on both Box A and B	33
3.15	Stress versus no. of mesh for plate ($t=1\text{mm}$) (Eigenvalue = 0.036138)	35
3.16	Non-dimensional load-end shortening variation	36
4.1	The mode shape of plate ($t=1\text{mm}$) at buckling condition	39
4.2	The mode shape of plate ($t=2\text{mm}$) at buckling condition	40
4.3	The mode shape of Box A (thickness ratio = 1) at buckling condition	41
4.4	The mode shape of Box B (thickness ratio = 2) at buckling condition	42
4.5	The comparison between stress over strain of plate ($t=1\text{m}$) and Box A	43
4.6	Comparison between stress over strain of plate ($t=1\text{mm}$), plate ($t=2\text{mm}$) and Box B	43
4.7	Stress distributions between Box A and plate ($t=1\text{mm}$) at location (a) Node, (b) Avg, (c) Crest in scales 1, 2 and 3	45
4.8	Stress distributions between Box B and plate ($t=1\text{mm}$) at location (a) Node, (b) Avg, (c) Crest in scales 1, 2 and 3	47
4.9	Stress distributions between Box B and plate ($t=2\text{mm}$) at location (a) Node, (b) Avg, (c) Crest in scales 1, 2 and 3	49

LIST OF ABBREVIATIONS

b	Width of plates
E	Young modulus
t	Thickness of plates
γ	Poisson's Ratio
P	Applied load
P_c	Critical load
\bar{u}	Applied end-shortening
L	Length of plate structure
D	Bending stiffness parameter
t_w	Thickness of web plate
t_f	Thickness of flange plate
σ_{cr}	Critical stress
δ	Displacement of plate
P^*	Non-dimensional load
u^*	Non dimensional displacement
P_c^*	Critical non-dimensional load
u_c^*	Critical non-dimensional displacement
λ	Eigenvalue
v	Eigenvector
node	Node location of a plate
avg	Average location of a plate
crest	Crest location of a plate

FEM	Finite Element Method
Plate (t=1mm)	Plate with thickness 1mm
Plate (t=2mm)	Plate with thickness 2mm
NODE A	Flange plate at node
NODE B	Web plate at node
AVG A	Flange plate at avg
AVG B	Web plate at avg
CREST A	Flange plate at crest
CREST B	Web plate at crest



CHAPTER 1

INTRODUCTION

This chapter described briefly the research conducted which consist of the background of the research, problem statements, objectives of the research, and scope of the research and thesis outline.

1.1 Research Background

Researchers and engineers were curious to study the issue of failure in structural elements through buckling, as this issue was rising and generating. The basic underlying physical action creating the buckling of axially compressed columns was the reduction of its lateral stiffness to a value of zero. Complication in the prediction of buckling loads in actual engineering situations were affected by a number of intricacies. However, these structures were always associated with failure in structural elements after subjected to heavy loads and lead to buckling. Apart from that, this was a common issue, hence had attracted researchers and engineers to understand and assess the reliability of complex structures through the study of the buckling behavior of prismatic columns (Ziółkowski and Imielowski, 2011).

Structures which were stiff in the loaded direction and slender in another direction were exposed to buckling phenomenon. Buckling loads were critical loads where certain kinds of structure become imbalance. As load was increased, there had been displacement in the slender direction causes by sudden increase in deflection in the loading direction. Besides, this happens at location that was initially stable in equilibrium and then known as bifurcation point. This point plays a crucial role for structural behavior to govern slender constructions and to reach plastic or elastic cross-sectional capacity. A structure was considered stable when there was no buckling at a certain load level (Bhoi and Kalurkar, 2014).

Some situations where wrinkling occurs at isolated location and some thin portions of the cross section buckle locally in compression were known as local buckling. Local buckling takes place before other modes of buckling occur and it may cause the cross section to be ineffective and the steel compression members to fail. For example, local buckling at thin flanges and web elements of an I-shaped cross section results in failure of column (Bhoi and Kalurkar, 2014). In addition, constituting elements that were small in thickness were then prone of getting flexural buckling. Excessive flexural deformation may also happen due to commonly used of built up and hot rolled steel compression members. Hence, this present study also included post-buckling response of the imperfect buckled structures.

There had been a few numerical simulation procedures and analytical studies done using the finite strip method of analysis, assuming that the plate components at the junctions were uncoupled and free to wave in their own plane along the junctions, while the plate's out-of-plane deflection disappears. However, these assumptions inflict stress-free in-plane conditions in the width direction of each plate and had violated the compatibility and equilibrium conditions along the junctions (Grave Smith, 1966). Hence, this present research work was to study the conditions of junctions between the flange and web plates of the cross sections along the lengths of the struts using the finite element method. This allows nodes at the section junction to be constraint-free and coupling of the in-plane and out-of-plane deflections of the section walls to happen and leads to waviness of the junctions along the length of the strut. Furthermore, this method had more ability to study the natural compatibility and equilibrium conditions at the junctions.

1.2 Problem Statement of the Research

Prismatic plates and plate structures were increasingly used as structural components in various branches of engineering, chief of which were aerospace and marine engineering. In aerospace particularly, the quest for efficient and light-weight structures often leads to risk of local buckling and post-local-buckling at design load levels. These structures were often subjected to in-plane compressive loading. Ovesy et al., (2006) had developed finite strip method to study post-local-buckling behavior of locally buckled I-beams with thin flanges. In addition, there were already many finite strip method developed by researchers such as Hancock (1981), Sridharan and Ali (1985 and 1986), and Graves Smith and Sridharan (1978) to study only post-local-buckling problems. Conversely, finite element method had larger number of degrees of freedom to give no limitations of boundary conditions and local discontinuities. It was a more dominant form of geometrically non-linear structural analysis and this present study gave considerable computational effort in structure discovery.

Loughlan et al., (2009) summarized that local buckling of the section walls can alter the compressional stiffness of thin-walled I-section strut. Cross-sectional shape and section all thickness affected the loss in axial stiffness of short strut compression members. The study further proved that high through-the-thickness bending stresses from local buckling and local imperfections were significant to cause simultaneous buckling yielding design (Loughlan et al., 2011). Hence, it was significant to study the effect local buckling and post-buckling in thin-walled box-sections with different thickness.

Rhodes (2002) illustrated using box-section tube with sides of unequal thickness that the rotational restraint was more significant at the point of buckling and in the initial stages of the post-buckling behavior. Thus, the author's study was to conduct the investigation on influences of the thin plate thickness towards the behavior of buckling and post-buckling of steel box-section struts under displacement. Appropriate thickness distribution resulted in substantial increase in critical local buckling loads of plates. Furthermore, his study was valuable as the analysis can help to come up with an economic design as plates of variable thickness were particularly used as aircraft wings and turbine discs.

In addition, the literature of study on the influence of thickness of thin plate towards behavior of buckling and post-buckling of steel box-section were least available. Most of the studies of square box-section struts in compression had been focusing on constant uniform thickness (Azhari et al., 2005). This was due to the sections manufactured, available in the market and the standard used by the manufactures to form the sections. All square and rectangular sections had been formed with wall thickness uniform around the section. Nevertheless, it was possible to form the square box-sections with different thickness ratios. This gave more flexibility in design and reduce wastages. Thus, this current work studied the behavior of box-section struts with different thickness ratios (i.e. $t_w/t_f = 1$ and 2) looking into the loading history and stress distributions during compression loading. Furthermore, the difficulties in determining the local buckling strength and behavior of such plates by using experimental method of analysis may be known as one of the reason for the problems. Therefore, a numerical method such as the finite element method would be the most suitable techniques for solving the problem based on the complication of the problems, such as lack of symmetry in the plate thickness and the axial load distributions.

1.3 Objectives of the Research

This research work was conducted by using the numerical method which was known as finite element simulation software, i.e. ABAQUS/CAE and had been chosen because of the simple and consistent interface for creating, monitoring and evaluating the results from the simulation. The main objective of this research was to investigate the influence of different plate thickness on the steel box-section strut buckling and post-buckling behavior. The objectives of this work were:

1. To develop non-linear finite element modelling strategy and solution procedures for the post-local-buckling analysis of the plates and box-section struts under uniform end shortening.
2. To verify and validate the developed finite element simulation.
3. To investigate the behavior of the stress distributions of the plates and box-section struts in the post-buckling region with regards to varying thickness ratios of the box-section struts.

1.4 Scopes of the Research

In order to achieve the objectives of the study, the understanding on basic buckling and post-buckling concept on steel box-section strut of different thickness ratio was conducted especially when subjected to axial loads. In current study, the method to conduct the analysis was by using the numerical method only, i.e. finite element analysis (FEA) software. In addition, this method was considered to be used in present study because it can save time to gain the results and saving cost. Since current study use the finite element analysis (FEA) software, i.e. ABAQUS/CAE to conduct the analysis, therefore the technique to use the software was learned.

In determining the best material for the current study, an investigation on the materials that had been used throughout the study was conducted through the literature study. The selection of the material for the study was based on the properties of the materials and its application that were widely used in recent industries especially in aerospace industry. The properties of the materials that had the highest demand in current industries was the material that poses high strength and light weight. Thus, the best material that had been selected to be used in the current study was steel due to its material properties and its typical application in aerospace industry.

Besides, there were two types of basic model geometry for current study, which were thin plate of 1mm thickness and thin plate of 2mm thickness. The size of the thin plate designed for current study was limited to 100mm x 100mm of length and width. The thin plates were constructed to a square box-section structure with different thickness ratio 1:1 and 1:2. Then, both geometry structure were analyzed by using finite element analysis software. The result predicted from the numerical analysis were verified and validated by referring to the existing theoretical data obtained from the literature study conducted. In order to ensure the results of the analysis was accurate and acceptable, thus the percentage difference considered for current study shall be below than 10%.

Finally, the results for all investigation conducted for current study was analyzed and discussed to investigate the behavior of the buckling and post-buckling of steel box-section with different thickness ratios when subjected to axial loads. In the meantime, the comparison between the behavior of buckling and post-buckling of box-section of thickness ratio 1:1 and 1:2 were determined. Lastly, the outcome for the study was concluded with some recommendation for future study.

1.5 Thesis Outline

The overall thesis outline contains five (5) chapters, and the content of these chapters are organized as following:

- Chapter 1: Introduction

This chapter generally summarizes the background of the research conducted on the buckling and post-buckling of square box-section.

- Chapter 2: Literature Review

This chapter is discussing the literature review based on the previous study conducted and published paper related with buckling and post-buckling of square box-section.

- Chapter 3: Methodology

This chapter highlights the method implemented in the research by using FEA software with the verification and validation of the numerical data obtained.

- Chapter 4: Results and Discussions

This chapter provides the results and some discussions of the research which relates to buckling and post-buckling behavior of square plates and square box when subjected to axial load.

- Chapter 5: Conclusions and recommendations

This chapter presents a comprehensive conclusion based on the findings of the research conducted and some recommendation for future works.

REFERENCES

- Azhari, M., Shahidi, A. R., & Saadatpour, M. M. (2005). Local and post local buckling of stepped and perforated thin plates. *Applied Mathematical Modelling*, vol. 29(7), pp. 633–652.
- Baiz, P. M., & Aliabadi, M. H. (2009). Local buckling of thin-walled structures by the boundary element method. *Engineering Analysis with Boundary Elements* vol. 33(3), pp. 302–313.
- Ban, H., Shi, G., Shi, Y., & Wang, Y. (2012). Overall buckling behavior of 460Mpa high strength steel columns: Experimental investigation and design method. *Journal of Constructional Steel Research*, vol. 74, pp. 140–150.
- Belytschko, T., Liu, W. K., Moran, B., & Elkhodary, K. (2000). Nonlinear finite elements for continua and structures. Wiley, New York.
- Bhoi, R., & Kalurkar, L. G. (2014). Study of buckling behaviour of beam and column subjected to axial loading for various rolled I-sections. *International Journal of Innovative Research in Science, Engineering and Technology*, vol. 3(11), pp. 17639–17645.
- Bjelajac, N. (2000). Evaluation of post-buckling equilibrium branches for perfect and imperfect plates by finite difference method. *Numerical Methods in continuum Mechanics, Liptovskoy Jaon, Slovak Republic*, pp. 1-18.
- Burgueno, R., Hu, N., Heeringa, A., & Lajnef, N. (2014). Tailoring the elastic post-buckling response of thin-walled cylindrical composite shells under axial compression. *Thin-Walled Structures*, vol. 84, pp. 14-25.
- Chaje, A. (1974). Principles of Structural Stability Theory, London: Prentice - Hall Inc.
- Clarín, M. (2004). High strength steel: local buckling and residual stresses. *Environmental Engineering*.
- Eisenberger, M., & Alexandrov, A. (2003). Buckling loads of variable thickness thin isotropic plates. *Thin-Walled Structures*, vol. 41(9), pp. 871–889.
- Elek, P. M., Jaramaz, S. S., Micković, D. M., & Miloradović, N. M. (2016). Experimental and numerical investigation of perforation of thin steel plates by deformable steel penetrators. *Thin-Walled Structures*, vol. 102, pp. 58–67.
- European Committee for Standardization. Eurocode 3: Design of steel structures — part 1-12: General - high strength steels, 28.10.2010
- European Convention for Constructional Steelwork. Committee 8, Stability. *Manual on Stability of Steel Structures*. Brussels, 2nd Edition, 1976.

- Gardner, L. & Nethercot, D. A. (2004). Experiments on stainless steel hollow sections – Part 2: Member behaviour of columns and beams, *Journal of Constructional Steel Research*, vol. 60 (9), pp. 1319–1332.
- Graves-Smith, T.R. & Sridharan, S. (1978). A finite strip method for the post-locally-buckled of plate structures. *Int. J. Mech. Sci.*, Vol. 20, pp. 833-842.
- Graves-Smith, T. R. (1966). The ultimate strength of locally buckled columns of arbitrary length. PhD Thesis, Fitzwilliam College, University of Cambridge.
- Guerrero, N., Marante, M. E., Picón, R., & Flórez-López, J. (2007). Model of local buckling in steel hollow structural elements subjected to biaxial bending. *Journal of Constructional Steel Research*, vol. 63(6), pp. 779–790.
- Hancock, G. J. (1981). Nonlinear analysis of thin-sections in compression. *Journal of the Structural Division. ASCE*, 107, No. ST3, March, pp. 455-71.
- Haran, B. (2009). Sixty Symbols. From eigenvalues: <http://sixtysymbols.com/videos/eigenvalues.htm>
- Hassan, M. N. (2018). Influence of stiffener on aluminium perforated plates subjected to in-plane shear loading. Msc Thesis. Universiti Putra Malaysia.
- Hermann, S. D. (2002). On the local stability interaction effect of high strength steel columns. *Steel Members and Structural Systems, Eurosteel Coimbra*, pp. 353-360.
- Hui-Shen, S., Pi, Z., & Tie-Yun, C. (1991). Buckling and post-buckling of stiffened cylindrical shells under axial compression. *Applied Mathematics and Mechanics*, vol. 12(12), pp. 1195-1207.
- Husain, M., Aldaami, H., & Ammash, H. (2004). Post-buckling Behavior of Thin Plates with Initial Curvature. *Tikrit Journal of Engineering Science*, vol. 70, pp. 33-42.
- Kala, Z., Kala, J., & Omishore, A. (2016). Probabilistic buckling analysis of thin-walled steel columns using shell finite elements. *International Journal of Mechanics*, vol. 10, pp. 213– 218.
- Kasai, A., Miyamoto, Y. Kawaoka, H. & Susanti, L. (2014). Numerical study on ultimate strain at critical segments in steel compression members. *Journal of Japan Society of Civil Engineers Ser. A2 (Applied Mechanics (AMI))*, vol. 70(2), pp. 575-586.
- Koiter, W. T. (1945). The stability of elastic equilibrium. Ph.D. Thesis, Delft University of Technology.
- Lagaros, N. D., & Papadopoulos, V. (2006). Optimum design of shell structures with random geometric, material and thickness imperfections. *International Journal of Solids and Structures*, vol. 43(22–23), pp. 6948–6964.

- Liu, E. L. & Wade, M. A. (2015). Interactively induced localization in thin-walled I-section struts buckling about the strong axis. *Structures*, vol. 4, pp. 13–26.
- Liu, Y., & Young, B. (2003). Buckling of stainless steel square hollow section compression members. *Journal of Constructional Steel Research*, vol. 59(2), pp. 165–177.
- Loughlan, J. (1996). Structural Stability Lecture Notes. Cranfield University, UK.
- Loughlan, J., Yidris, N., & Cunningham, P. R. (2009). The influence of post-local buckling mechanics on the stress variations, axial stiffness and ultimate failure strength of uniformly compressed thin-walled I-Section struts. *Journal of Physics: Conference Series*, 181(1).
- Loughlan, J., Yidris, N., & Cunningham, P. R. (2011). The effects of local buckling and material yielding on the axial stiffness and failure of uniformly compressed I-section and box-section struts. *Thin-Walled Structures*, vol. 49(2), pp. 264–279.
- Magnucki, K., & Milecki, S. (2017). Elastic buckling of a thin-walled rectangular frame under in-plane compression. *Thin-Walled Structures*, vol. 116, pp. 326–332.
- Mohammed, A. M. (2001). Effect of initially imperfect and boundary condition on the buckling and post-buckling behavior of steel plates. M. Sc. Thesis, Department of Building and Construction, University of Technology, Iraq.
- Nishino, F., Ueda, Y., & Tall, L. (1967). Experimental investigation of the buckling of plates with residual stresses. *Tests methods for compression members, American Society for Testing and Materials*, vol. 419, pp. 12- 30.
- Oguaghamba, O. A., Ezeh, J. C., & Ibearugbulem, M. O. (2015). Buckling and post-buckling loads characteristics of all edges clamped thin rectangular plate. *The International Journal of Engineering and Science*, vol. 4, pp. 55-61.
- Ovesy, H. R., Loughlan, J., & GhannadPour S. A. M. (2006). Geometric non-linear analysis of channel sections under end shortening, using different versions of the finite strip method. *Computers and Structures*, vol. 84, pp. 855-872.
- Pavlovčič, L., Froschmeier, B., Kuhlmann, U., & Beg, D. (2012). Finite element simulation of slender thin-walled box columns by implementing real initial conditions. *Advances in Engineering Software*, vol. 44(1), 63–74.
- Rasmussen, K. J. R. & Hancock, G. J. (1992). Plate slenderness limits for high strength steel sections. *Journal of Constructional Steel Research*, vol. 23(1-3), pp. 73-96.
- Rhodes J. (2002). Buckling of thin plates and members and early work on rectangular tubes. *Thin-Walled Structures*, vol. 40, pp. 87–108.
- Rhodes, J. (2003). Some observations on the post-buckling behavior of thin-walled members. *Thin-Walled Structures*, vol. 41, pp. 207 – 26.

- Ruocco, E. (2015). Elastoplastic buckling analysis of thin-walled structures. *Aerospace Science and Technology*, vol. 43, pp. 176–190.
- Rushton, K. R. (1969). Post-buckling of tapered plates. *Int. J. Mech. Sci.*, vol.11, pp. 461-480.
- Schillo, N. (2017). Local and global buckling of box columns made of high strength steel. M. Sc. Thesis, Diss. RWTH Aachen University, Germany.
- Schillo, N., & Feldmann, M. (2015). Local buckling behaviour of welded box sections made of high-strength steel. *Steel Construction*, vol. 8(3), pp. 179–186.
- Shen, J., & Wadee, M. A. (2018). Length effects on interactive buckling in thin-walled rectangular hollow section struts. *Thin-Walled Structures*, vol. 128, pp. 152–170.
- Shen, J., Wadee, M. A., & Sadowski, A. J. (2017). Interactive buckling in long thin-walled rectangular hollow section struts. *International Journal of Non-Linear Mechanics*, vol. 89, 43–58.
- Sherbourne, A. N., & Korol, R. M., (1972). Post-buckling of axially compressed plates. *J. Struc. Div.*, vol. 98, pp. 2223-2234.
- Simuleon. (2015). Retrieved June 25, 2021, from Post-buckling analysis: <https://www.simuleon.com/post-buckling-analysis/>
- Smith-Pardo, J. P. (1999). Buckling reversals of axially restrained imperfect beam-column. *Journal of Engineering Mechanics*, vol. 125(4), pp. 401-409.
- Sridharan, S. & Ali, M. A. (1985). Interactive buckling in thin-walled beam columns. *Journal of Engineering Mechanics. ASCE*, vol. 111(12), pp. 1470-86.
- Sridharan, S. & Ali, M. A. (1986). An improved interactive buckling analysis of thin-walled columns having double symmetric cross-sections. *Int. J. Solid and Struct.*, vol. 22(4), pp. 429-43.
- Susanti, L., Kasai, A., & Miyamoto, Y. (2014). Ultimate strength of box section steel bridge compression members in comparison with specifications. *Case Studies in Structural Engineering*, vol. 2, pp. 16-23.
- Susanti, L., Kasai, A., and Miyamoto, Y. (2015). Postbuckling behaviour of welded box section steel compression members. *Inter. Journal of Civil Engineering and Technology*, vol.6, pp. 65-78.
- Tan, M., & Cheng, W. (2020). Nonlinear buckling analysis of thin-walled box beams considering shear lag. *Mathematical Problems in Engineering*.
- Tang, L. R. B. & Mahendran, M. (2004). Behavior of high strength steel compression members. *Proc. 10th Nordic Steel Construction Conference*, Copenhagen, Denmark, pp. 53-64.

- Usami, T. & Fukumoto, Y. (1982). Local and overall buckling of welded box columns. *Journal of the Structural Division, Proceedings of the American Society of Civil Engineers*, vol. 108(3), pp. 525–542.
- Usami, T. & Fukumoto, Y. (1984). Welded box compression members. *Journal of Structural Engineering*, vol. 10, pp. 2457–2470.
- Usami, T. (1982). Post-buckling of plates in compression and bending. *Journal of Structural Engineering*, vol. 108, pp. 591–609.
- Van der Neut, A. (1969). The interaction of local buckling and column failure of thin-walled compression members, in: *Proceedings of the 12th International Congress on Applied Mechanics*, Springer, 1969, pp. 389–399.
- Wadee, M. A. & Bai, L. (2014). Cellular buckling in I-section struts. *Thin-Walled Structures* vol. 81, pp. 89–100.
- Wadee, M. A. & Farsi, M. (2014). Cellular buckling in stiffened plates. *Proceedings of the Royal Society A* 470 (2168) 20140094
- Wadee, M. A., Gardner, L. & Osofero, A. I. (2013). Design of prestressed stayed columns. *J. Construct. Steel Research*, vol. 80, pp. 287–98.
- Walker, A. C. (1969). The post-buckling of simply-supported square plates. *The Aeronautical Quarterly*, vol. 20, pp. 203–222.
- Wang, J., Schillo, N., & Theofanous, M. (2017). Material properties and compressive local buckling response of high strength steel square and rectangular hollow sections. *Engineering Structures* vol. 130, pp. 297–315.
- Wang, X., & Wang, Y. (2015). Buckling analysis of thin rectangular plates under uniaxial or biaxial compressive point loads by the differential quadrature method. *International Journal of Mechanical Sciences*, vol. 101–102, pp. 38–48.
- Wang, Y., Li, G., and Chen, S (2012). The assessment of residual stresses in welded high strength steel box sections. *Journal of Constructional Steel Research*, vol. 76, pp. 93–99.
- Xu, H. (2013). (TEZ) Buckling, Postbuckling and Imperfection Sensitivity Analysis of Different Type of Cylindrical Shells by Hui's Postbuckling Method. 189
- Yamaki, N. (1959). Postbuckling behaviour of rectangular plates with small initial curvature loaded in edge compression. *Journal of Applied Mechanics*, vol. 26, pp. 407–414.
- Yang, D. & Hancock, G. J. (2004). Compression tests of cold-reduced high strength steel channel columns failing in the interaction between local and distortional modes. *Journal of Structural Engineering*, vol. 130, No 12, pp. 1954–1963.

- Yang, D., & Hancock, G. J. (2006). Numerical Simulation of High-Strength Steel Box-Shaped Columns Failing in Local and Overall Buckling Modes. *Journal of Structural Engineering*, 132(4), 541–549.
- Yang, L., Shi, G., Zhao, M., & Zhou, W. (2017). Research on interactive buckling behavior of welded steel box-section columns. *Thin-Walled Structures*, vol. 115, pp. 34–47.
- Yingjiang, Z., Renjun, Y., & Hongxu, W. (2015). Experimental and numerical investigations on plate girders with perforated web under axial compression and bending moment. *Thin-Walled Structures*, vol. 97, pp. 199–206.
- Zhang, T., Li, S., Chang, F., Shi, X., & Li, L. (2016). An experimental and numerical analysis for stiffened composite panel subjected to shear loading in hygrothermal environment. *Composite Structures*, 138, 107–115.
- Zhang, T., Li, S., Chang, F., Shi, X., & Li, L. (2016). An experimental and numerical analysis for stiffened composite panel subjected to shear loading in hygrothermal environment. *Composite Structures*, vol. 138, pp. 107–115.
- Zhou, F., Chen, Z., Zheng, C., Xu, F., Hu, Y., Hou, S., & Qin, Z. (2017). Experimental and numerical buckling failure analysis of acrylic hemispheres for application in neutrino detector. *Engineering Failure Analysis*, vol. 78, pp. 147–160.
- Ziółkowski, A., & Imiełowski, S. (2011). Buckling and post-buckling behaviour of prismatic aluminium columns submitted to a series of compressive loads. *Experimental Mechanics*, vol. 51(8), pp. 1335–1345.

BIODATA OF STUDENT

Nur Hazwani binti Isham was born on 9th December 1994 at Kuantan, Pahang. She graduated her Bachelor Degree in Aerospace Engineering from Universiti Putra Malaysia in 2018. In the same year, she furthers her postgraduate study in Master of Science in Aerospace Engineering at Universiti Putra Malaysia. She was selected for Graduate Research Fellowship (GRF) from Universiti Putra Malaysia which get sponsored for study fees and monthly allowance for two semesters.



PUBLICATION

Journal

Yidris, N., Isham, N. H., Gires, F., & Mutafi, A. (2020). The effects of loading conditions on the behavior of fixed-ended plain channel columns. *Structure Analysis and Characterizations. Materials 2020, 13(6)*, pp. 1441.





UNIVERSITI PUTRA MALAYSIA
STATUS CONFIRMATION FOR THESIS / PROJECT REPORT AND COPYRIGHT
ACADEMIC SESSION: Second Semester 2020/2021

TITLE OF THESIS / PROJECT REPORT:

BUCKLING AND POST-BUCKLING OF STEEL BOX SECTION USING FINITE ELEMENT METHOD

NAME OF STUDENT: NUR HAZWANI BINTI ISHAM

I acknowledge that the copyright and other intellectual property in the thesis/project report belonged to Universiti Putra Malaysia and I agree to allow this thesis/project report to be placed at the library under the following terms:

1. This thesis/project report is the property of Universiti Putra Malaysia.
2. The library of Universiti Putra Malaysia has the right to make copies for educational purposes only.
3. The library of Universiti Putra Malaysia is allowed to make copies of this thesis for academic exchange.

I declare that this thesis is classified as:

*Please tick (✓)

CONFIDENTIAL

(Contain confidential information under Official Secret Act 1972).

RESTRICTED

(Contains restricted information as specified by the organization/institution where research was done).

OPEN ACCESS

I agree that my thesis/project report to be published as hard copy or online open access.

This thesis is submitted for:

PATENT

Embargo from _____ until _____
(date) (date)

Approved by:

(Signature of Student)
New IC No/ Passport No.:

(Signature of Chairman of Supervisory Committee)
Name:

Date:

Date:

[Note: If the thesis is CONFIDENTIAL or RESTRICTED, please attach with the letter from the organization/institution with period and reasons for confidentially or restricted.]