

UNIVERSITI PUTRA MALAYSIA

MODELLING WATER AND SEDIMENT FLOW IN BRANCHING CHANNEL SYSTEM

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By

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Thesis Submitted to the School of Graduate Studies, Universiti Putra Malaysia, in Fulfilment of the Requirements for the Degree of Doctor of Philosophy

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DEDICATION

То

My parents



Abstract of the thesis presented to the Senate of Universiti Putra Malaysia in fulfilment of the requirement for the degree of Doctor of Philosophy

MODELLING WATER AND SEDIMENT FLOW IN BRANCHING CHANNEL SYSTEM

By

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May 2017

Chairman: Badronnisa Binti Yusuf, PhD Faculty: Engineering

Operational problems associated with branching channels and water intakes increase over time due to localised sediment accumulations. The success of branching channel projects depends on the right design to increase the unit discharge and decrease the sediment concentration as much as possible. In addition, a branching channel from rivers and channels affects the bed morphology and causes erosion and sedimentation in the branching junction. There is still a lack of studies on the flow pattern in movable bed branching channel systems for different branching channel angles and bed widths to quantify the amount of water and sediment concentration in the branching channel and investigate its effect on the bed morphology. In this study, the hydraulic performance of differently angled branching channels was compared in an effort to maximise discharge, minimise sediment concentration and decrease its effect on the bed morphology.

The objectives of the study are to investigate the effect of the branching angle and the bed width ratio on the water and sediment flow in the branching channel and scour hole characteristics (scour depth and scour length). The scour hole is formed in the main channel just downstream from the location of the branching channel entrance. This study also investigated the variation in velocity vertically and horizontally at the junction region and determined the total energy loss coefficient across the junction region. The objectives of the study were implemented experimentally using a physical model of 30, 45, 60, 75 and 90° branching channel angles with the main flow direction. In addition, three bed width ratios (30, 40 and 50%) and five total discharges (7.25, 8.5, 9.75, 11, and 12.25 L/s) were investigated for each branching angle scenario. A sand bed with d_{50} of 0.4 mm was used for all the experiments. In order to ensure sediment movement in the main channel at the upstream and to quantify the branching channel sediment concentration, a live-bed condition with flow intensity (V_u/V_c) of 1.1–1.5 was maintained in all experiments.

The results indicated that branching angles of 30° and 45° increased the relative discharge ratios (Q_R) by approximately 5–10% compared with the discharge ratio for the 90° branching angle. The results also indicated that the branching channel sediment concentration and scour depth decreased as the branching channel angle decreased. The branching angles of 30° and 45° reduced the sediment concentrations by an average of 64% and 37%, respectively, compared with the concentration for the 90° branching angle. With respect to scour depth, the branching angle of 30° reduced the scour depth for the 90° branching angle. The main reasons for forming the scour depth for the 90° branching angle. The main reasons for forming the branching channel and the downstream branching channel entrance sharp edge.

The 30° branching angle recorded the smaller low velocity region at the beginning of the upstream side wall of the branching channel than other branching angles. Moreover, the velocity distribution in this branching angle is more uniform along the branching channel width than others. The outcomes from this study indicate that a branching angle of 30° – 45° is the best arrangement to increase the branching channel discharge, decrease the branching channel sediment concentration and decrease the scour depth at the junction region. Reducing the amount of branching channel flow. In addition, a high water unit discharge means a lower initial construction cost for the channel. Moreover, decreasing the scour depth helps to reduce the risks of a scouring effect on the side bank of the main channel or any nearby structures.

Abstrak tesis yang dikemukakan kepada Senat Universiti Putra Malaysia sebagai memenuhi keperluan untuk ijazah Doktor Falsafah

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Masalah operasi yang berkaitan dengan saluran bercabang dan tempat masuk air meningkat dengan masa kerana pengumpulan enapan setempat. Kejayaan projek saluran bercabang bergantung kepada reka bentuk yang untuk yang meningkatkan unit kadar alir dan mengurangkan kepekatan enapan sebanyak mungkin. Tambahan lagi, saluran bercabang dari sungai dan saluran menjejas morfologi dasar sungai dan menyebabkan hakisan dan pemendapan di simpang pencabangan. Kajian mengenai corak aliran di dalam sistem pencabangan saluran dasar sungai bergerak bagi sudut pencabangan saluran dan lebar dasar sungai yang berbeza untuk menyatakan kuantiti jumlah air dan kepekatan enapan di dalam saluran bercabang dan menyiasat kesannya ke atas morfologi dasar sungai masih berkurangan. Di dalam kajian ini, prestasi hidraulik saluran pencabangan bersudut berbeza telah dibandingkan dalam usaha untuk memaksimumkan kadar alir, meminimumkan kepekatan enapan dan mengurangkan kesannya ke atas morfologi dasar sungai.



Objektif kajian ini adalah untuk mengkaji kesan sudut pencabangan dan nisbah lebar dasar sungai ke atas aliran air dan enapan di dalam saluran bercabang dan ciri-ciri lubang kerukan (kerukan mendalam dan kerukan panjang). Lubang kerukan terbentuk di dalam saluran utama di hilir berdekatan dengan lokasi masuknya saluran yang bercabang. Kajian ini juga menyiasat perubahan halaju secara menegak dan mendatar di kawasan simpang dan menentukan pekali kehilangan tenaga merentasi kawasan persimpangan tersebut. Objektif kajian ini telah dilaksanakan secara eksperimen dengan menggunakan model fizikal sistem pencabangan saluran 30, 45, 60, 75 dan 90° dari arah aliran utama. Di samping itu, tiga nisbah lebar dasar sungai (30, 40 dan 50%) dan lima jumlah kadar alir (7.25, 8.5, 9.75, 11, dan 12.25 L/s) telah disiasat untuk setiap senario sudut pencabangan. Satu dasar sungai pasir dengan d₅₀ 0.4 mm digunakan untuk semua eksperimen. Untuk memastikan pergerakan enapan di hulu saluran utama dan untuk mendapatkan kuantiti kepekatan enapan

saluran, keadaan dasar sungai hidup dengan keamatan aliran (V_u/V_c) antara 1.1 -1.5 dikekalkan di dalam semua eksperimen.

Keputusan menunjukkan bahawa sudut pencabangan 30° dan 45° meningkatkan nisbah kadar alir relatif (Q_R) sebanyak kira-kira 5-10% berbanding dengan nisbah kadar alir untuk sudut pencabangan 90° . Keputusan juga menunjukkan bahawa kepekatan enapan di cabang saluran dan kedalaman kerukan menurun apabila sudut pencabangan saluran berkurangan. Sudut pencabangan 30° dan 45° mengurangkan kepekatan enapan secara purata sebanyak 64% dan 37%, masing-masing, berbanding dengan kepekatan bagi sudut pencabangan 90° . Berkenaan dengan kedalaman kerukan, sudut pencabangan 30° mengurangkan kedalaman kerukan sebanyak kira-kira 14.4-46.7% berbanding dengan kedalam kerokan bagi sudut pencabangan 90° . Sebab-sebab utama pembentukan lubang kerukan adalah putaran yang dijanakan kerana pengalihan sebahagian aliran ke arah saluran yang bercabang dan sisi tajam laluan masuk di hilir saluran cabang.

Sudut pencabangan 30° mencatatkan kawasan halaju rendah yang libih kecil pada awal dinding hulu saluran bercabang di berbanding sudut-sudut yang lain. Selain itu, agihan halaju untuk sudut pencabangan ini adalah lebih seragam di sepanjang kelebaran saluran bercabang itu berbanding yang lain. Hasil daripada kajian ini mengesyorkan bahawa sudut pencabangan di antara 30°- 45° ialah susunan terbaik untuk meningkatkan kadar alir saluran bercabang, mengurangkan kepekatan enapan saluran bercabang dan mengurangkan kedalaman kerukan di kawasan persimpangan. Mengurangkan jumlah enapan saluran bercabang mengekalkan kecekapan projek yang, bergantung pada aliran saluran ber cabang. Tambahan lagi, kadar alir unit air yang tinggi bermakna lebih kurang kos awal pembinaan saluran. Lebih-lebih lagi, mengurangkan kedalaman kerukan membantu mengurangkan risiko kesan kerukan ke atas tebing sisi saluran utama atau mana-mana struktur berdekatan.

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TABLE OF CONTENTS

	Page
ABSTRACT	i
ABSTRAK	iii
ACKNOWLEDGEMENTS	V
APPROVAL	vi
DECLARATION	viii
LIST OF TABLES	xiii
LIST OF FIGURES	XV
LIST OF ABBREVIATIONS	xxii
CHAPTER	

1	INTI	RODUCTION	1
	1.1	Background	1
	1.2	Problem Statement	4
	1.3	Objectives of the Study	5
	1.4	Significance of the Study	6
	1.5	Scope and Limitations	7
	1.6	Thesis Layout	7
2	LITI	ERATURE REVIEW	9
	2.1	Introduction	9
	2.2	Flow Characteristics of the Branching Channel	9
		2.2.1 Branching Channel Water Discharge	9
		2.2.2 Water Depths, Water Surface and Hydraulic Jump	14
		2.2.3 Velocity Distribution and Streamlines	16
		2.2.4 Separation Zones	18
		2.2.5 Stagnation Point	20
		2.2.6 Contraction Coefficient	21
		2.2.7 Flow Power, Energy Head and Energy Loss Coefficients	
		2.2.8 Vortex Regions	22
		2.2.9 Pressure Recovery Factor	24
	2.3	Physical Characteristics of the Branching Channel	24
		2.3.1 Branching Angle	24
		2.3.2 Bed Slope	25
		2.3.3 Bed Roughness	25
	2.4	Sediment Transport in the Branching Channel	26
		2.4.1 Erosion and Sedimentation Regions	27
		2.4.2 Sediment Control at the Branching Channel Flow	28
	2.5	Case Studies of Branching Channel Practical Applications	33
	2.6	Modelling of the Branching Channel	34
		2.6.1 Physical Models	34
		2.6.2 Mathematical and Numerical Models	35
	2.7	Summary of the Literature Review	40
3	MAT	FERIALS AND METHODS	42
	3.1	Introduction	42

(6)

3.2	Dimensional Analysis	42
	3.2.1 Dimensional Analysis of the Branching Channel Water	40
	Flow	42
	3.2.2 Dimensional Analysis of the Branching Channel Sedime Flow	ent 43
	3.2.3 Dimensional Analysis of the Scour Depth and Scour Lei	ngth
	y i	44
	3.2.4 Dimensional Analysis of the Energy loss coefficient	45
3.3		46
0.0	3.3.1 Branching Channel System Flume	46
	3.3.2 Bed Material	51
	3.3.3 Re-circulation System	53
	3.3.4 Laboratory Measurement	54
3.4	•	59
3.5	-	62
3.6		66
3.0	-	67
5.7		07
4 RE	SULTS AND DISCUSSIONS	70
4.1	Introduction	70
4.2	Branching Channel Water Discharge	70
4.3	Branching Channel Sediment Discharge	81
	4.3.1 Sediment Flow Field at the Main and Branching Channe	el
	Junction	81
	4.3.2 Branching Angle and Bed Width Ratio Effect on Branch	ning
	Channel Sediment Concentration	88
4.4		92
	4.4.1 Effect of the Branching Angle and Bed Width Ratio on t	
	Scour Depth	98
	4.4.2 Effect of the Water Depth on the Scour Depth	109
	4.4.3 Effect of the Velocity on the Scour Depth	112
	4.4.4 Effect of the Total Discharge on the Scour Depth	115
	4.4.5 Estimation of the Scour Depth and Scour Length in the	110
	Branching Channel Flow	116
4.5	Velocity Distribution at the Junction Region	119
	4.5.1 Contours of Velocity at the Plan	120
	4.5.2 Contours of Velocity at Cross-sections	125
4.6		131
т.0	4.6.1 Total Energy Loss Coefficient	132
	4.6.2 Estimation of the Total Energy Loss Coefficient in the	152
	Branching Channel Flow	137
	Dranching Chainer Plow	157
5 SU	MMARY, CONCLUSIONS AND RECOMMENDATIONS	139
5.1	Summary	139
5.2	Conclusions	140
5.3		141
5.4	5	142

xi

REFERENCES	143
APPENDICES	155
BIODATA OF STUDENT	169
LIST OF PUBLICATIONS	170



LIST OF TABLES

	Table		Page
	2.1	Typical Physical Models Properties, Which were Used to Simulate Branching Channel Flow.	36
	3.1	Results of Testing of the Three Sand Samples.	53
	4.1	Summary of Experimental Data for a Branching Angle (θ) of 30°.	71
	4.2	Summary of Experimental Data for a Branching Angle (θ) of 45°.	72
	4.3	Summary of Experimental Data for a Branching Angle (θ) of 60°.	73
	4.4	Summary of Experimental Data for a Branching Angle (θ) of 75°.	74
	4.5	Summary of Experimental Data for a Branching Angle (θ) of 90°.	75
	4.6	Relative Discharge Ratio (Q_R) As a Function of Branching Angle (θ) for Different Bed Width Ratios (B_r) .	77
	4.7	Relative Unit Discharge Ratio (q_R) As a Function of Branching Angle (θ) for Different Bed Width Ratios (B_r) .	80
	4.8	Average Branching Channel Sediment Concentrations (C_{gb}) and Relative Concentration Reductions (Rel C_{gb} Red, with a 90° Branching Angle Baseline) for Different Branching Angles (θ), Bed Width Ratios (B_r), and Main Channel Discharges at Upstream (Q_r).	90
	4.9	Constants, Exponents and Coefficient of Determination (R^2) Values Used for the Relationship between d_s/B_b and θ for Different Flow Conditions and Branching Channel Bed Width Ratios.	101
	4.10	Scour Depth Values and Percentage of Decrement Compared to 90° Branching Angle.	102
	4.11	Relative Scour Depth (K_{ds}) Values for Different Branching Angles and Bed Width Ratios.	105
	4.12	Constants, Exponents and (R^2) Values Used for the Relationship between (d_s/B_b) and (y_u/B_b) for Different Branching Angles and Bed Width Ratios.	111
	4.13	Constants, Exponents and (R^2) Values Used for the Relationship between (d_s/B_b) and (V_u/V_c) for Different Branching Angles and Bed Width Ratios.	114

4.14	Increase in Scour Depth Because of Increase in Total Discharge Compared to Scour Depth at 7.25 L/s (%).	116
4.15	Scour Length Values for Different Cases of the Study.	117
4.16	Constants (a ₄) and Coefficient of Determination (R ²) Values for Different Branching Angles and Bed Width ratios.	134



LIST OF FIGURES

Figure		Page
1.1	Patia River Bifurcation (Casas, 2013).	1
1.2	Natural Branching Channel Flow from the Cumberland Marshes, Canada.	2
1.3	Lateral Water Intake from the Ohio River (Neary et al., 1999).	3
1.4	Branching Channel through the West Bank of the Mississippi River (Miller, 2004).	3
2.1	Relationship between (F_{rd}) and (Q_r) (Hsu et al., 2002).	10
2.2	Relationship between (F_{rd}) with (Q_r) (Ramamurthy & Satish, 1988).	12
2.3	(Ø) Function for Trapezoidal Main Channel as a Function of the Specific Energy (Cheong, 1991).	14
2.4	Water Surface Profiles for $(Q_r = 83.8\%)$: (a) Experimental Data; (b) Numerical Model, Where, X [*] , Y [*] and Z [*] are the Coordinate System Normalised by the Channel Bed Width (Ramamurthy et al., 2007).	15
2.5	Relationships Between (y_u/y_d) , (F_{rd}) and (Q_d/Q_u) (Hsu et al., 2002).	16
2.6	3D Flow Pattern in the Branching Channel Flow (Neary et al., 1999).	18
2.7	Velocity-Vector and Streamlines Patterns ($V_r = 0.6$): (a) Rough Bed; (b) Smooth Bed (Neary & Odgaard, 1993).	19
2.8	Separation Zones and Stagnation Point in the Branching Channel System (Ramamurthy et al., 2007).	20
2.9	Vortex and Recirculation Regions at the junction Region: (V) Vortex Region; (R) Recirculation Region (Casas, 2013).	23
2.10	Velocity Profiles in Main Channel (Neary & Odgaard, 1993).	26
2.11	Erosion and Sedimentation Regions Observed by Casas (2013).	28
2.12	Vane Arrangement Schemes at the Branching Channel Entrance Investigated by Barkdoll et al. (1999).	29
2.13	Upstream Interception Layout of Vane Arrangement Investigated by Barkdoll et al. (1999).	30

2.14	Single Row of Vanes Arrangement Investigated by AbdelHaleem et al. (2008).	30
2.15	Regular Vanes Arrangement Investigated by Allahyonesi et al. (2008).	31
2.16	Zigzag Vanes Arrangement Investigated by Allahyonesi et al. (2008).	31
2.17	Vanes Arrangement: (a) Parallel Arrangement; (b) Zigzag Arrangement Investigated by Moghadam and Keshavarzi (2010).	32
2.18	Recommended Design for Boom by Kubit and Ettema (2001).	33
3.1	Branching Channel Flume in Hydraulic Laboratory of Universiti Putra Malaysia.	47
3.2	Schematic of the Physical Model's Main and the Branching Channel Flume.	48
3.3	Schematic Plan and Side View Drawing of the Branching Channel System Flume.	49
3.4	Main and Secondary Pumps, Flow Meter and Control Valve.	49
3.5	Tooth-Shaped Perspex Acrylic Flow Contractors Installed in the Main Channel.	50
3.6	Flattening of the Bed Material.	51
3.7	Sand Preparation and Distribution Curve.	52
3.8	Determination of the Specific Gravity of the Bed Material in the Laboratory.	53
3.9	Schematic Plan View of the Re-circulation System.	55
3.10	Re-circulation System: (a) Filter in the Branching Channel Outlet Tank; (b) Filter in the Main Channel Outlet Tank; and (c) Main and Secondary Pumps with Their Inlets and Outlets.	55
3.11	Using Sluice Gate to Measure Water Discharge in the Main Channel at Downstream, Which was Installed at the End of the Main Channel.	56
3.12	Determination of the Sluice Gate Coefficient.	57
3.13	Pitot Tube and Point Gauge with Carriage.	57
3.14	Sediment Basket and Measurement: (a) Main Channel Sediment Basket; (b) Sediment Collection from the Branching Channel Outlet.	58

3.15	Using Dye to Detect the Dividing Streamlines and the Vortexes.	58
3.16	Hjulstrom's Diagram, by Hickin (1995).	60
3.17	Bed Material Critical Velocity as a Function of the Water Depth.	61
3.18	Measured Scour Depth and Water Depth Locations.	63
3.19	Scour Depths at Points A, B and C ($\theta = 90^{\circ}$, $B_r = 50$, $Q_u = 12.25$ L/s).	64
3.20	Scour Depths at Points A, B and C ($\theta = 90^{\circ}$, B _r = 50, Q _u = 7.25 L/s).	64
3.21	Branching Channel Sediment Discharge Concentration (ppm), ($\theta = 90^{\circ}$, $B_r = 50$, $Q_u = 7.25$ and 12.25 L/s).	65
3.22	Main Channel Sediment Discharge Concentration at Downstream (ppm), ($\theta = 90^{\circ}$, $B_r = 50$, $Q_u = 7.25$ and 12.25 L/s).	65
3.23	Experimental Procedures.	66
3.24	Water Surface out of Phase with Bed Surface: (a) at the Beginning of the Experiment; (b) During the Experiment.	68
3.25	Locations of the Measurement Velocity at Cross-sections.	69
4.1	Average Discharge Ratio (Q_r) As a Function of Branching Angle (θ) for Different Bed Width Ratios (B_r).	76
4.2	Branching Channel Unit Discharge (q_b) As a Function of Upstream Main Channel Unit Discharge (q_u) for Different Bed Width Ratios (B_r) and a Branching Angle (θ) of 30°.	78
4.3	Branching Channel Unit Discharge (q_b) As a Function of Upstream Main Channel Unit Discharge (q_u) for Different Bed Width Ratios (B_r) and a Branching Angle (θ) of 45°.	78
4.4	Branching Channel Unit Discharge (q_b) As a Function of Upstream Main Channel Unit Discharge (q_u) for Different Bed Width Ratios (B_r) and a Branching Angle (θ) of 60° .	79
4.5	Branching Channel Unit Discharge (q_b) As a Function of Upstream Main Channel Unit Discharge (q_u) for Different Bed Width Ratios (B_r) and a Branching Angle (θ) of 75°.	79
4.6	Branching Channel Unit Discharge (q_b) As a Function of Upstream Main Channel Unit Discharge (q_u) for Different Bed Width Ratios (B_r) and a Branching Angle (θ) of 90°.	80
4.7	Dyed Streamlines in the Main Channel Flow Layers at the Junction Region: (a) Near the Water Surface; (b) Near the	82

Channel Bed; (c) Near the Channel Bed at the Middle of the Main Channel Width.

- 4.8 Typical Observed Bed Topography and vortexes at the Junction 83 Region for a Branching Angle (θ) of 60°, A Bed Width Ratio (B_r) of 40%, and Main Channel Discharge at Upstream(Q_u) of 9.75.
- 4.9 Initial Set of Vortexes (V1) Generated at the Maximum Scour 84 Depth Location.
- 4.10 Cross-Sectional View of Vortex Effects for Different Branching Angles (θ), a bed Width Ratio (B_r) of 30%, and an Upstream Main Channel Discharge (Q_u) of 9.75 L/s (Cross-section was Taken Parallel to the Main Channel 3 cm from the Branching Side Wall).
- 4.11 Cross-Sectional View of Vortex Effects for Different Branching 85 Angles (θ), a bed Width Ratio (B_r) of 40%, and an Upstream Main Channel Discharge (Q_u) of 9.75 L/s (Cross-section was Taken Parallel to the Main Channel 3 cm from the Branching Side Wall).
- 4.12 Cross-Sectional View of Vortex Effects for Different Branching 85 Angles (θ), a bed Width Ratio (B_r) of 50%, and an Upstream Main Channel Discharge (Q_u) of 9.75 L/s (Cross-section was Taken Parallel to the Main Channel 3 cm from the Branching Side Wall).
- 4.13 Cross-Sectional View of Vortex Effects for Different Upstream 86 Main Channel Discharges (Q_u), a Bed Width Ratio (B_r) of 40%, and a Branching Angle (θ) of 30° (Cross-section was Taken Parallel to the Main Channel 3 cm from the Branching Side Wall).
- 4.14 Cross-Sectional View of Vortex Effects for Different Upstream 86 Main Channel Discharges (Q_u), a Bed Width Ratio (B_r) of 40%, and a Branching Angle (θ) of 45° (Cross-section was Taken Parallel to the Main Channel 3 cm from the Branching Side Wall).
- 4.15 Cross-Sectional View of Vortex Effects for Different Upstream 87 Main Channel Discharges (Q_u) , a Bed Width Ratio (B_r) of 40%, and a Branching Angle (θ) of 60° (Cross-section was Taken Parallel to the Main Channel 3 cm from the Branching Side Wall).
- 4.16 Cross-Sectional View of Vortex Effects for Different Upstream 87 Main Channel Discharges (Q_u) , a Bed Width Ratio (B_r) of 40%, and a Branching Angle (θ) of 75° (Cross-section was Taken Parallel to the Main Channel 3 cm from the Branching Side Wall).
- 4.17 Cross-Sectional View of Vortex Effects for Different Upstream 88 Main Channel Discharges (Q_u) , a Bed Width Ratio (B_r) of 40%, and a Branching Angle (θ) of 90° (Cross-section was Taken Parallel to the Main Channel 3 cm from the Branching Side Wall).
- 4.18 Branching Channel Sediment Concentration over Time for 89 Different Branching Angles (θ), a Bed Width Ratio (B_r) of 40%, and Main Channel Discharge at Upstream (Q_r) of 12.25 L/s.

	4.19	Formation of the Scour during the Experiment.	93
	4.20	Scour Depth: (a) during the Experiment; (b) at the End of the Experiment.	94
	4.21	Bed Topography at the Junction Region for All Branching Angles, $(B_r = 30\%, Q_u = 7.25 \text{ L/s}).$	94
	4.22	Bed Topography at the Junction Region for All Branching Angles, $(B_r = 40\%, Q_u = 7.25 \text{ L/s}).$	95
	4.23	Bed Topography at the Junction Region for All Branching Angles, $(B_r = 50\%, Q_u = 7.25 \text{ L/s}).$	95
	4.24	Bed Topography at the Junction Region for All Branching Angles, ($B_r = 30\%$, $Q_u = 12.25$ L/s).	96
	4.25	Bed Topography at the Junction Region for All Branching Angles, ($B_r = 40\%$, $Q_u = 12.25$ L/s).	96
	4.26	Bed Topography at the Junction Region for All Branching Angles, $(B_r = 50\%, Q_u = 12.25 \text{ L/s}).$	97
	4.27	Timeline of the Scour Depth at Different Branching Angles ($B_r = 40\%$, $Q_u = 7.25$ L/S and $Q_r = (24.6 \pm 1.8)$ %).	97
	4.28	Effect of Branching Angles on the Normalised Scour Depth for Different Total Discharges and $B_r = 30\%$.	99
	4.29	Effect of Branching Angles on the Normalised Scour Depth for Different Total Discharges and $B_r = 40\%$.	99
	4.30	Effect of Branching Angles on the Normalised Scour Depth for Different Total Discharges and $B_r = 50\%$.	100
	4.31	Relative Scour Depth (K_{ds}) for Different Branching Angles and Bed Width Ratios.	104
	4.32	Variation in the Normalised Scour Depth with the Bed Width Ratio for Branching Angle of 30°.	106
	4.33	Variation in the Normalised Scour Depth with the Bed Width Ratio for Branching Angle of 45°.	107
	4.34	Variation in the Normalised Scour Depth with the Bed Width Ratio for Branching Angle of 60° .	107
	4.35	Variation in the Normalised Scour Depth with the Bed Width Ratio for Branching Angle of 75°.	108
	4.36	Variation in the Normalised Scour Depth with the Bed Width Ratio for Branching Angle of 90°.	108
	4.37	Normalised Scour Depth as a Function of the Normalised Water Depth at the Upstream ($B_r = 30\%$).	109

	4.38	Normalised Scour Depth as a Function of the Normalised Water Depth at the Upstream ($B_r = 40\%$).	110
	4.39	Normalised Scour Depth as a Function of the Normalised Water Depth at the Upstream ($B_r = 50\%$).	110
	4.40	Normalised Scour Depth as a Function of the Flow Intensity, V_u/V_c (B _r = 30%).	112
	4.41	Normalised Scour Depth as a Function of the Flow Intensity, V_u/V_c (B _r = 40%).	113
	4.42	Normalised Scour Depth as a Function of the Flow Intensity, V_u/V_c (B _r = 50%).	113
	4.43	Timeline of the Scour Depth for a Range of Total Discharges ($\theta = 60^{\circ}$, $B_r = 40^{\circ}$, $Q_r = (25.5 \pm 0.4)$ %).	115
	4.44	Comparison of Estimated and Measured Data for Scour Depths.	118
	4.45	Comparison of Estimated and Measured Data for Scour Lengths.	119
	4.46	Contours of Velocity at the Plan for Branching Angle of 45°, Bed Width Ratio of 50% and Main Channel Water Discharge at Upstream of 11 L/s.	120
	4.47	Contours of Velocity at the Plan for Different Branching Angles, a Bed Width Ratio of 30% and a Main Channel Water Discharge at Upstream of 7.25 L/s.	122
	4.48	Contours of Velocity at the Plan for Different Branching Angles, a Bed Width Ratio of 40% and a Main Channel Water Discharge at Upstream of 7.25 L/s.	122
	4.49	Contours of Velocity at the Plan for Different Branching Angles, a Bed Width Ratio of 50% and a Main Channel Water Discharge at Upstream of 7.25 L/s.	123
	4.50	Contours of Velocity at the Plan for Different Branching Angles, a Bed Width Ratio of 30% and a Main Channel Water Discharge at Upstream of 12.25 L/s.	123
	4.51	Contours of Velocity at the Plan for Different Branching Angles, a Bed Width Ratio of 40% and a Main Channel Water Discharge at Upstream of 12.25 L/s.	124
	4.52	Contours of Velocity at the Plan for Different Branching Angles, a Bed Width Ratio of 50% and a Main Channel Water Discharge at Upstream of 12.25 L/s.	124
	4.53	Cross-section Velocities at the Junction Region ($\theta = 30^{\circ}$, $Q_u = 12.25$ L/s, $B_r = 40\%$): (a) Main Channel at Upstream; (b) Main Channel in Front of Branching Channel Entrance; (c) Main Channel at Downstream; (d) Branching Channel.	126

4.54	Cross-section Velocities at the Junction Region ($\theta = 45^{\circ}$, $Q_u = 12.25$ L/s, $B_r = 40\%$): (a) Main Channel at Upstream; (b) Main Channel in Front of Branching Channel Entrance; (c) Main Channel at Downstream; (d) Branching Channel.	127
4.55	Cross-section Velocities at the Junction Region ($\theta = 60^{\circ}$, $Q_u = 12.25$ L/s, $B_r = 40\%$): (a) Main Channel at Upstream; (b) Main Channel in Front of Branching Channel Entrance; (c) Main Channel at Downstream; (d) Branching Channel.	128
4.56	Cross-section Velocities at the Junction Region ($\theta = 75^{\circ}$, $Q_u = 12.25$ L/s, $B_r = 40\%$): (a) Main Channel at Upstream; (b) Main Channel in Front of Branching Channel Entrance; (c) Main Channel at Downstream; (d) Branching Channel.	129
4.57	Cross-section Velocities at the Junction Region ($\theta = 90^{\circ}$, $Q_u = 12.25$ L/s, $B_r = 40\%$): (a) Main Channel at Upstream; (b) Main Channel in Front of Branching Channel Entrance; (c) Main Channel at Downstream; (d) Branching Channel.	130
4.58	Estimating K_e Values for Different Branching Angles and a Bed Width Ratio of 30%.	133
4.59	Estimating K _e Values for Different Branching Angles and a Bed Width Ratio of 40%.	133
4.60	Estimating K _e Values for Different Branching Angles and a Bed Width Ratio of 50%.	134
4.61	Comparison of Ke Values for Different Branching Angles and Bed Width Ratios.	135
4.62	Relation between Main Channel Power at Downstream to Upstream Ratio (P_d/P_u) and Main Channel Discharge at Downstream to Upstream Ratio (Q_d/Q_u) Power Loss.	136
4.63	Relation between Power Ratio and Discharge Ratio.	136
4.64	Comparison of Estimated and Measured Data for Total Energy Loss Coefficient in the Branching Channel Flow.	138
C . 1	Sand Particles Separated According to Sizes.	161
C. 2	Preparation the Sand for Channel Bed According to Designed Sand Distribution Carve.	162
C. 3	Fabrication of the End Part (with Drain Hole) of the Branching Channel.	162
C. 4	Fabrication of Branching Channel and its Connection to the Main Channel.	163
C. 5	Testing the Branching Channel Flow System Flume.	163

C. 6	Installing the Rails Along the Top of the Branching Channel	164
	Walls to Hold the Point Gauge and the Pitot Tube with Their	
	Carriage.	

C. 7	Laying Sand on the Branching Channel Flow System Flume.	164
C. 8	Changing the Branching Channel Bed Width from 15cm to 12cm.	165
C. 9	Branching Channel Flow System with Branching Angle of 30°.	166
C. 10	Branching Channel Flow System with Branching Angle of 45°.	166
C. 11	Branching Channel Flow System with Branching Angle of 60°.	167
C. 12	Branching Channel Flow System with Branching Angle of 75°.	167

C. 13 Branching Channel Flow System with Branching Angle of 90°. 168

LIST OF ABBREVIATIONS

	а	Constant of the equation
	А	Coefficient from experimental data depending on total discharge from 0 to 1 in Equation (2.16)
	b	Exponent of the equation
	B _b	Bed widths of the branching channel
	Be	Width of the branching channel entrance
	B _m	Bed widths of the main channel
	Br	Bed width ratio = $B_{b/}B_m$
	c	Weir crest shape effect
	Cc	Flow contraction coefficient in the branching channel
	C _d	Sluice gate coefficient
	C_{gb}	Mass sediment concentration in the branching channel
	C_{gd}	Mass sediment concentration in the main channel at the downstream
	C_m	Coefficient of the branching discharge over side weir
	C_{sb}	Volumetric sediment concentration in the branching channel
	d ₅₀	Medium particles diameter
	ds	Depth of scour
	F*	Dimensionless shear stresses or Shield's parameter
	Frb	Froude number in the branching channel
	F _{rd}	Froude number in the main channel at the downstream
	Fru	Froude number in the main channel at the upstream
	g	gravity acceleration
	Н	Total energy head
	H_b	Total energy head in the branching channel
	H_d	Total energy head in the main channel at downstream
	H_r	Total energy head ratio = H_b/H_u

xxiii

	H_u	Total energy head in the main channel at upstream
	K ₁₂	Energy loss coefficient between main channel at the upstream and at the downstream
	K ₁₃	Energy loss coefficient between main channel at the upstream and branching channel
	K _{ds}	Relative scour depth = $d_{s (\theta^{\circ})} / d_{s (90^{\circ})}$
	Ke	Total energy loss coefficient
	L	Side weir opening length
	1	Length of the vortex
	Ls	Length of scour
	m	Side slope of the main channel
	ma	Mass involved in the vortexes
	n*	Number of the side weir
	P _b	Flow power of the branching channel
	P _d	Flow power in the main channel at downstream
	Pr	Power ratio = $P_{b/} P_{u}$
	P _{rd}	Pressure force in the main channel at the downstream conjunction edge
	P _{rds}	Pressure force at the downstream branching channel wall
	P _{ru}	Pressure force in the main channel at the upstream conjunction edge
	P _{rus}	Pressure force at the upstream branching channel wall
	Pu	Flow power in the main channel at upstream
	q	Unit discharge
	Qb	Water discharge in the branching channel
	q _b	Water unit discharge in the branching channel
	Q_d	Water discharge in the main channel at the downstream
	q_d	Water unit discharge in the main channel at the downstream
	Qr	Discharge ratio = Q_b/Q_u
	qr	Unit discharge ratio = q_b/q_u
	Q _R	Relative discharge ratio

	q _R	Relative unit discharge ratio compared with the discharge ratio for the 90° branching angle
	q_{sb}	Sediment unit discharge in the branching channel
	Q_{sb}	Volumetric sediment discharge in the branching channel
	Q_{sbt}	Bed main channel sediment discharge at upstream
	q _{sd}	Sediment unit discharge in the main channel at the downstream
	Qslu	Discharge under the sluice gate
	Q _{sst}	Suspended main channel sediment discharge at upstream
	Qsu	Volumetric sediment discharge in the main channel at upstream
	Qt	Total discharge
	Qu	Water discharge in the main channel at the upstream $= Q_t$
	qu	Water unit discharge in the main channel at the upstream $= q_t$
	R	Recirculation Region
	\mathbb{R}^2	Coefficient of determination
	R _c	Static pressure recovery coefficient
	R _e	Reynold's number
	r _{sb}	Bed sediment ratio
	r _{ss}	Suspended sediment ratio
	Ry	Main channel water depth at the upstream to the downstream = y_u/y_d
	Sb	Slope of the branching channel
	Sg	Specific gravity of the sand
	S _m	Slope of the main channel
	Sr	Branching to main sediment ratio
	Т	Water temperature
	U	Velocity of the vortex
	U^{*}	Shear velocity
	U_{c}^{*}	Critical shear velocity of the particles inception motion
	V	Vortex

XXV

	V_b	Velocity of the flow in the branching channel
	Vc	Critical velocity of the particles inception motion
	\mathbf{V}_{d}	Velocity of the flow in the main channel at the downstream
	V_p	Local flow velocity measured by Pitot tube
	Vr	Velocity ratio = V_b/V_u
	V_u	Velocity of the flow in the main channel at the upstream
	V _x	Velocity component towards the main flow
	Vy	Velocity component towards the branching flow
	Vz	Velocity component normal to the main flow
	W	Height of the side weir
	W/C	Water content
	W _P	Weight of the pycnometer
	W _{PS}	Weight of the pycnometer + dry soil
	W _{PSW}	Weight of the pycnometer + dry soil + water
	W_{PW}	Weight of the pycnometer + water
	X	Flow meter reading of the main pump
	у	Water depth
	y1	Water depth before the sluice gate
	y 2	water depth under the sluice gate
	Уb	Water depth in the branching channel
	Yd	Water depth in the main channel at the downstream
	y _r	Water depth ratio = y_b/y_u
	Yu	Water depth in the main channel at the upstream
	Z_b	Bed elevation in the branching channel
	Zd	Bed elevation in the main channel at downstream
	Zu	Bed elevation in the main channel at upstream
	φ	Main channel contraction angle

xxvi

β_d	Downstream momentum correction factor
β_u	Upstream momentum correction factor
γ	Water weight density = ρg
γ_s	Bed materials weight density
δ	Strength of the secondary circulation
Δh	Dynamic head = difference between total and static head at the Pitot tube submerged head
θ	Branching angle
	Functions depend on the main channel geometry, Equation (2.8)
ρ	Water density
ρs	Bed material density
σ_{g}	Bed material standard deviation
υ	Water viscosity

C

CHAPTER 1

INTRODUCTION

1.1 Background

Open channels in irrigation and drainage projects are the most important means to transport water. In many cases, channels need to branch out to secondary channels so that it can feed lateral projects, such as irrigation and the water supply of municipality plants and hydro-power projects (Al Omari, 2009). Therefore, studying the branching channel flow system has a direct application in water resource projects (Meselhe et al., 2016; Ramamurthy et al., 1990). Moreover, branching channel flow or river bifurcations are found in natural rivers as a result of the rivers' dynamics processes (Kleinhans et al., 2013; Redolfi, 2015; Redolfi et al., 2016).

There are many examples of branching channels or river bifurcations in nature, such as the Pannerdense Kop in the lower course of the Rhine River and the bifurcations in the Patia River delta in Colombia (Figure 1.1). Another example of branching flow is the branching of two channels from the Cumberland Marshes, Canada, as shown in Figure 1.2.



Figure 1.1: Patia River Bifurcation (Casas, 2013).



Figure 1.2: Natural Branching Channel Flow from the Cumberland Marshes, Canada.

Man-made branching channels from a channel or river are considered as practical applications of the branching channel flow system. These branching channels vary in their objectives, such as supplying water for the cooling systems of an energy or power station, supplying municipal water, supplying water for irrigation projects, or for any other objectives.

Figure 1.3 shows an example of the branching channel from the Ohio River to supply water to a lateral project (Neary et al., 1999). Other examples of lateral projects, which are supplied with water from the Missouri River in North America by branching channels, are the MidAmerican Energy Company's Council Bluffs Power Station, the Omaha Public Power District-Nebraska City Station, the St. Louis County Water Company to supply municipal water, and the MidAmerican Energy Company's George Neal Station water intake.

Branching channel systems can also be used to capture a portion of the main flow sediment particles (sediment diversion) and subsequently decrease the main flow sediment concentration (Brown et al., 2013; Meselhe et al., 2016).

Miller (2004) gave another example of the practical application of branching channel flow. This is represented by designing and constructing a branching channel through the west bank of the Mississippi River. The branching channel is used to restore the wetlands located in Plaquemines Parish by connecting the Mississippi river with the wetlands (Figure 1.4). This branching channel carries water, sediment and nutrients from the river to the wetlands.



Figure 1.3: Lateral Water Intake from the Ohio River (Neary et al., 1999).





Figure 1.4: Branching Channel through the West Bank of the Mississippi River (Miller, 2004).

Branching channel flow has been studied in recent decades e.g. (Bulle, 1926; Grace & Priest, 1958; Taylor, 1944) and still attracts the attention of water resources engineering researchers as it commonly exists in many water engineering related projects and depends on many variables. Due to the complexity of branching flow,

involving many interlinked factors, generalisation of the phenomena involved is very difficult to achieve (Lama et al., 2002).

1.2 Problem Statement

The problem of the branching sediment flow always accompanies the branching channel system (Moradinejad et al., 2017; Raudkivi, 1993). Over time, branching channels may experience a decrease in their hydraulic efficiency due to an accumulation of sediment particles, for instance, by building up the sediment within the branching channel bed from the Ohio River, as shown in Figure 1.3 (Neary et al., 1999). The sediment concentration in the branching channel is higher than the concentration in the main channel. The branching channel receives more water from the main channel's lower layers where the sediment particles are concentrated (Barkdoll, 2004). The main channel's upper layers have higher momentum and tend to continue downstream past the branching channel while the lower momentum in lower layers are easily diverted to the branching channel (Herrero et al., 2015; Neary & Odgaard, 1993; Omidbeigi et al., 2009).

The sediment particles may block filters and damage water pumps in the water supply and hydropower systems. In addition, irrigation network systems, especially when these systems depend on irrigation by drops or sprinkler systems, always need sediment-free water to prevent their nozzles from blockage or damage. Rehabilitation efforts often require the accumulated sediment to be dredged from the branching channels at a high cost.

The mechanisms of the sediment transport in river bifurcations are still not well understood. Therefore, attempts of managing the branching channel sediment flow are still considered as a challenge (Sassi et al., 2013). Researchers have previously investigated various strategies for reducing sediment in branching channels, with a focus on using submerged vanes in front of the entrance to the branching channel (Beygipoor et al., 2013; Michell et al., 2006; Neill et al., 1997; Yalin Wang et al., 1996). The use of submerged vanes to control and redirect the upstream sediment downstream in the main channel is potentially costly and may adversely affect navigation of the main channel (Barkdoll et al., 1999). While, designing the branching channels and water intakes for municipal use should be stable and maintain the navigation of the main channel (Kleinhans et al., 2013). Moreover, it may inadvertently direct ice to the branching channel (Yushi Wang et al., 2014). The effectiveness of the submerged vanes is also limited based on flow conditions (effective approach for a unit discharge ratio up to 20-30%). The limitations of using submerged vanes to control the branching channel sediment flow display the importance of investigating alternative approach to control and manage of the branching channel sediment by choosing an appropriate branching angle, which can effectively reduce the branching channel sediment discharge.

Although bed morphology is considered as an essential element of the design of a branching channel (Xu et al., 2016), most of the studies related to branching channel flow have been carried out with the rigid boundary condition, e.g. (Mignot et al., 2014;

Mignot et al., 2013; Momplot et al., 2017), while most of the branching channel flow studies with a movable bed condition focused only on branching channel flow with a branching angle of 90°, e.g. (Barkdoll et al., 1999; Herrero et al., 2015). Al Omari (2009) and Casas (2013) reported that the effect of the branching angle of 30° and 60° on sediment transport in the branching channel system is lacking and should be taken into account in a future studies.

In addition, the branching channel can affect the main channel flow, alter the channel bed mechanics and change the bed morphology. The change of the bed topography and the turbulence of the flow in the region of the branching channel junction, besides the boundary friction, contribute to the energy loss. Furthermore, the branching channel leads to the formation of erosion and sedimentation regions at the branching channel junction (Allahyonesi et al., 2008). Some researchers have reported the formation of scour downstream of the channel junction (Barkdoll et al., 1999; Casas, 2013). Up to now, to our knowledge, no explicit studies investigating the effect of the branching channel flow with different branching angles and bed width ratios on the scour depth and the scour length have been done.

Therefore, there is a real need to investigate the branching channel discharge and sediment concentration, with the movable bed condition, for a wide range of branching angles and different bed width ratios.

Quantifying branching channel water discharge and sediment concentration and investigating the characteristics of the scour hole for different scenarios of branching angle and bed width ratio help to find the best arrangement of the branching channel that can increase the discharge and decrease the sediment concentration in the branching channel.

1.3 Objectives of the Study

The main objective of this study is to investigate experimentally the water and sediment flow and bed morphology in a branching channel system to compare the hydraulic performance of differently angled branching channels in an effort to maximise discharge, minimise sediment concentration, and reduce scour hole. The specific objectives of this study can be listed as follows:

- 1. To quantify the water and sediment flow in a branching channel and to compare the results for various branching angles and bed width ratios.
- 2. To investigate the characteristics of the scour hole (scour depth and scour length) forming at downstream of the main channel due to the branching channel system.
- 3. To investigate the variation in velocity vertically and horizontally at the junction region of the branching channel system.
- 4. To determine the total energy loss coefficient across the junction region in the branching channel system.

1.4 Significance of the Study

This study provides more insight on the effect of a wide range of branching angles and bed width ratios on the water and sediment flow in branching channels, as well as on the scour depth, scour length, velocity distribution and total energy loss coefficient.

The findings are useful in designing and managing channel systems with branching channels. Studying the flow behaviour in the branching channel and flow diversion location is important for water management (Yousefi et al., 2011) and for sedimentation management downstream of the diversion (Baker et al., 2011).

In designing and managing branching channel systems, water resources engineers strive to maximise the water discharge while minimising the cross-sectional area and associated construction and operation costs. In hydropower projects for example, the generated energy amount has a direct relationship with the water discharge (Lazzaro et al., 2013). In systems with high sediment laden, the main challenge is to decrease the sediment entering the branching channel while maximising the branching channel water discharge. A high sediment concentration leads to an increment in the operation cost and a decrement in operational efficiency. For example, in the water supplying plants, a higher concentration of sediment means a higher filtration cost. The findings from this study offer an alternative approach in sediment control in a branching channel system through appropriate selection of branching angles and bed width ratios.

Most of the previous studies on branching channels were limited to branching channel flow with rigid boundary and branching angle of 90°, despite the facts that the natural branching channel systems have always occurred with movable boundary and at various branching angles depending on the bank stability relative to the flow strength (Kleinhans et al., 2013). Therefore, this study on the branching channel with movable bed and at various angles is significant as it provides better understanding of the complex branching channel system in the nature.

In a branching channel system, the formations of scours and sedimentations are also the main concerns as they affect the stability of the main channel banks and any crossing structures if any. Knowledge of the locations and the characteristics of scours helps engineers to select a suitable conjunction location, which is at a safe distance from other nearby hydraulic structures. In a case if a crossing bridge accompanying the branching junction area is required to connect roads on the different banks, the scour location and depth due to branching flow are important factors that need to be taken into account in the design of piers for the bridge besides the scour results from the pier itself.

Moreover, the study gives comprehensive understandings on the velocity distribution and total energy loss coefficient at the junction region at a much wider range of branching angles and bed width ratios. The velocity distribution provides essential information on the vortexes and turbulence characteristics at the channel junction.



1.5 Scope and Limitations

The scope of this research is to investigate experimentally a branching channel flow system with a sand bed condition. The experimental work includes five branching angles $(30^\circ, 45^\circ, 60^\circ, 75^\circ \text{ and } 90^\circ)$, three branching to main channel bed width ratios (30%, 40% and 50%) and five different total discharges for each case (7.25 L/s, 8.5 L/s, 9.75 L/s, 11 L/s and 12.25 L/s). The datasets are used to quantify and compare the different cases in terms of the branching channel water and sediment flow, scour depth, velocity distribution and total energy loss coefficient.

There are many variables that can be taken into account in the investigation of branching channel flow, but it is difficult to consider all of them. In this study, one type of bed material, sand soil with a medium diameter of 0.4 mm, specific gravity of 2.53 and standard deviation of 1.46 is used. Water and sediment are re-circulated during the experiment by collecting water at the ends of both the main and branching channels and pumping it to the upstream main channel and re-circulating again. A live bed load condition of the main channel at upstream with a velocity to threshold velocity of the sand movement of 1.1–1.5 is considered during the experiments to get an appropriate amount of sediment load into the branching channel to measure. The flow is subcritical with a Froude number around 0.3 for all the experiments.

Investigation the velocity distribution horizontally and vertically is limited only to highlight the flow-dividing pattern and the low and high velocity region not for developing a velocity model. The model scale is limited according to the available main channel flume, 12.5 m long and 0.313 m wide, and the space available for the fabricated branching channel flume, 2.75 m long, in the hydraulic laboratory at the Faculty of Engineering, Universiti Putra Malaysia.

1.6 Thesis Layout

This thesis is composed of five chapters. Chapter One, as shown above, presents a background about branching channel flow and its applications, the problems accompanying the flow phenomena, and the objectives of the study, together with the significance, scope and limitations of the current research.

Relevant literature is reviewed in Chapter Two. This Chapter extensively reviews the flow characteristics and the variables governing the flow in branching channels, the effect of the branching channel's physical characteristics, such as the branching angle and bed slope, and the sediment transport behaviour in the branching channel flow system. The Chapter also reviews some case studies of practical applications of branching channels and previous modelling types, dimensions and specifications of branching channel flow. Finally, there is a summary of the literature review and the research gaps related to branching channel flow studies.

Dimensional analysis of the parameters governing the branching channel water discharge, sediment discharge, the scour depth, scour length and the total energy loss coefficient in the branching flow system are demonstrated in the third Chapter. Chapter Three also includes a description of the physical model of the branching channel system, the preparation of the experimental work, facilities, data collection, calibration of laboratory apparatus and the programme of experiments.

The results of the experiments are presented and discussed as graphs, tables and / or empirical equations for each objective of the study individually in Chapter Four. Chapter Four also includes a comparison between different cases and their effect on the flow phenomena. Finally, Chapter Five presents a summary and conclusions of the study, as well as suggestions for some work in the future.


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