



**UNIVERSITI PUTRA MALAYSIA**

***SIMULATION OF CRACKS PROPAGATION OF REINFORCED  
CONCRETE BEAM-TO-COLUMN JOINT WITH FRP STRENGTHENING  
IN FLEXURAL AND SHEAR REGION***

**SHAHRIAR SHAHBAZPANAHI**

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**By**

**SHAHRIAR SHAHBAZPANAHI**

**Thesis Submitted to the School of Graduate Studies, Universiti Putra  
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Philosophy**

**November 2015**

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## **DEDICATION**

This research is dedicated to my lovely wife, Alaleh. I also want to dedicate this thesis to my father, mother and brothers for their supports.



Abstract of thesis presented to the Senate of Universiti Putra Malaysia in fulfilment of the requirement for the degree of Doctor of Philosophy

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**November 2015**

**Chairman : Professor Abang Abdullah Abang Ali, PhD**  
**Faculty : Engineering**

Nowadays, rehabilitation of structural members is a challenging issue for structural engineers, and much effort has been made to predict crack propagation in structural members. In the present study, a stiffness matrix is formulated for the Fracture Process Zone (FPZ). Based on the derived formulation, a new element was developed in order to model crack propagation using finite element analysis. Size effects such as depth, thickness of the beam and effective crack length were considered in the calculation of FPZ length and crack extension. Based on the new element, the Griffith differential energy method was developed to predict the crack propagation criterion with high accuracy.

Therefore, in the present investigation a numerical model was developed to model crack propagation in concrete beams flexural or shear strengthened with FRP. To validate the present model, experimental testing on reinforced concrete beams and beam-column joints with and without Fiber Reinforced Polymers (FRPs) strengthening were carried out. Three beam specimens with rectangular cross-section were tested. Two beams were strengthened with externally bonded FRP sheets for flexure or shear strengthening and one control beam were considered. One beam was externally bonded with FRP sheet at the bottom of the beam and another one was bonded with FRP sheet in the shear span i.e. the two sides of the beam. The beams were subjected to two point loads and tested to failure. The experimental results were compared to the present model predictions based on conventional fracture models carried out using commercial finite element software (ABAQUS). The results indicated that the use of FRP composites for flexural and shear-strengthened beams decreased crack propagation for approximately 55% and 37%, respectively, in comparison to the control beam. It was observed that the length of FPZ increased by using of FRP for shear-strengthening. The present model showed that the main diagonal crack formed at the support in the control beam whereas it appeared through the shear span in the shear-strengthened beam.

The developed fracture mechanics modeling was also applicable for identifying crack propagation in FRP-strengthened beam column joints. For this purpose, two beam column joints were made and tested to validate the present model. The results of the FRP-strengthened beam column joints by using present study showed good agreement with the experimental results (7 to 11%), whereas the results from numerical analysis using finite element software were considerably greater than experimental results (16 to 20 %). The results revealed that cracks formed in the joint area in the control specimen, while extensive cracks appeared in the beam in the specimen strengthened by FRP.



Abstrak tesis yang dikemukakan kepada Senat Universiti Putra Malaysia sebagai memenuhi keperluan untuk ijazah Doktor Falsafah

## **SIMULASI PEMANJANGAN REKAHAN DALAM SENDI RASUK-TIANG KONKRIT BERTETULANG YANG DIPERKUKUHKAN DENGAN FRP DI BAHAGIAN LENTURAN DAN RICIH**

By

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**Disember 2015**

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Pada masa kini, pemulihan bahagian-bahagian struktur merupakan isu yang mencabar untuk jurutera awam (struktural) dan usaha telah dipergiatkan untuk meramal pemanjangan rekahan dalam bahagian-bahagian struktur. Dalam kajian ini, sebuah matriks ketegaran telah dihasilkan untuk zon proses rekahan (FPZ). Berdasarkan formulasi yang didapati, satu elemen baru telah dibentuk untuk pemodelan pemanjangan rekahan menggunakan analisa elemen terhingga. Kesan-kesan saiz seperti kedalaman, ketebalan alang dan kepanjangan rekahan telah dipertimbangkan dalam pengiraan kepanjangan FPZ dan pelanjutan rekahan. Berdasarkan elemen baru itu, kaedah pembezaan tenaga Griffith telah digunakan untuk meramal ciri-ciri pemanjangan rekahan dengan ketepatan yang tinggi.

yang diperkuatkan dengan pelbagai kaedah seperti polimer yang diperkukuhkan dengan gentian (FRPs).

Oleh itu, dalam kajian ini, sebuah model yang berasaskan angka telah dihasilkan untuk pemodelan pemanjangan rekahan dalam alang konkrit yang diperkukuhkan dari segi kelenturan atau ricihan dengan FRP. Bagi mengesahkan ketepatan model ini, ujikaji eksperimen keatas rasuk dan sambungan rasuk-tiang konkrit bertetulang yang tidak diperkukuh dan yang diperkukuh dengan FRP dijalankan. Tiga spesimen rasuk dengan keratan rentas segi empat tepat telah diuji. Dua rasuk diperkukuhkan dengan kepingan FRP yang dilekat di luar untuk pengukuhan kelenturan atau ricihan dan satu rasuk sebagai sebagai kawalan. Pada salah satu rasuk, kepingan FRP diikat bersama secara luaran di bawah rasuk dan pada rasuk yang lain, kepingan FRP diikat pada rentang ricihan iaitu kepada 2 sisi rasuk. Rasuk tersebut dikenakan dua beban terpumpun sehingga gagal. Keputusan eksperimen telah diperbandingkan dengan ramalan model sedia ada berdasarkan keputusan ujian dengan model rekahan konvensional yang dijalankan menggunakan perisian elemen terhingga komersil (ABAQUS). Keputusan menunjukkan bahawa penggunaan komposit FRP untuk alang yang diperkukuhkan dari segi kelenturan dan ricihan telah mengurangkan pemanjangan rekahan sebanyak kira-kira 55% dan 37% masing-masing berbanding

dengan alang kawalan. Kapanjangan FPZ didapati bertambah dengan penggunaan FRP untuk pengukuhan kelenturan. Model ini menunjukkan bahawa rekahan pepenjuru utama terhasil di penyokong alang kawalan manakala terbentuk di kawasan ricihan dalam alang yang diperkukuhkan secara ricihan.

Pemodelan mekanik rekahan yang dihasilkan ini boleh digunakan untuk mengenalpasti pemanjangan rekahan dalam sendi rasuk-tiang yang diperkukuhkan dengan FRP. Untuk penggunaan sedemikian tujuan tersebut, dua sendi rasuk-tiang dihasilkan dan diuji untuk mengesahkan ketepatan model ini. Keputusan kajian ini adalah selaras dengan keputusan eksperimental (7 hingga 11%), manakala keputusan dari analisa berasaskan angka menggunakan perisian elemen terhingga didapati lebih besar dari keputusan eksperimental (16 hingga 20%). Keputusan menunjukkan bahawa rekahan terbentuk di bahagian sendi untuk spesimen kawalan manakala rekahan yang luas terbentuk di alang spesimen yang diperkukuhkan dengan FRP.



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You will always be in my heart.

This thesis was submitted to the Senate of Universiti Putra Malaysia and has been accepted as fulfillment of the requirement for the degree of Doctor of Philosophy. The members of the Supervisory Committee were as follows:

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## **Declaration by Members of Supervisory Committee**

This is to confirm that:

- the research conducted and the writing of this thesis was under our supervision;
- supervision responsibilities as stated in the Universiti Putra Malaysia (Graduate Studies) Rules 2003 (Revision 2012-2013) were adhered to.

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## LIST OF NOMENCLATURES

$P$	Applied load
$h$	Beam depth
$L$	Beam length
$B$	Beam width
$f'_c$	Compressive strength of concrete
$n$	Constant values
$\delta$	Crack extension
$u$	Crack opening displacement
$w_c$	Critical crack opening displacement
$G_c$	Critical strain energy release rate for concrete
$j$	Degrees of freedom
$u_1, u_2$	Displacement node 1 to the local coordinates $x$ and $y$
$u_3, u_4$	Displacement node 2 to the local coordinates $x$ and $y$
$u_5, u_7$	Displacement node in $x$ direction for nodes 3 and 4
$u_9, u_{11}$	Displacement node in $x$ direction for nodes 5 and 6
$U$	Displacement vector
$d^i$	Displacement, the corresponding increment after iteration $i$
$q(x)$	Distribution of stress in FPZ
$a$	Effective crack length
$\epsilon_c$	Effective strain for FRP sheet
$d_e, d_s$	Elastic opening and softening opening
$\dot{D}_p$	Energy dissipation rate by the flexural– FRP
$\dot{D}_s$	Energy dissipation rate by the shear– FRP (side bonding)
$G_{Ic}$	Energy release rate of concrete for Mode I
$K_{IC}$	Fracture resistance of the material
$K_{sII}$	Fracture toughness due to force and slip of FRP sheet for Mode II
$K_{cII}$	Fracture toughness of plain concrete and FPZ for Mode II
$A_F$	FRP area section
$f(a/h)$	Geometric shape function
$\tau(a/h)$	Geometric shape function due to the shear stress of the FPZ

$i$	Iteration
$l_{com}$	Length of the FPZ in composite material
$l_p$	Length of FRP
$a_0$	Length of initial notch
$\sigma$	Length stress-free region
$u$	Maximum crack opening displacement
$s_m$	Maximum of the slip between concrete and FRP
$\max$	Maximum shear stress between concrete and FRP
	Mesh size
	Modulus of composite
	Nodal force due to external load in $x$ direction
	Nodal force due to strain energy in $x$ direction
	Normal force due to the FPZ
$\sigma$	Normal stress in the FPZ
	Number of elements that have failed behind FPZ
$n'$	Number of FRP elements with no existing record of rupture
$n$	Number of side face FRP sheets
$\delta_n$	Opening displacement
	Orientation angle
$\nu_c$	Poisson's ratio of concrete
	Poisson's ratio of P
$s$	Shear force due to FRP sheet slip
	Shear modulus
$V_n$	Shear strength of the four-point loading RC beam
$V_{c, P}$	Shear strength contributions of the concrete and FPZ
	Shear stress between concrete and FRP
$\tau_{FPZ}$	Shear stress distributions in the damage zone
$\delta_t$	Slide displacement
$s$	Slip between concrete and FRP
$S_e, S_s$	Slopes in the elastic and softening zones
$S$	Softening parameter
$\Delta d^i$	Specified tolerance
$\mathbf{K}$	Stiffness matrix for local element



$\mathbf{K}_s$	Stiffness matrix for the element
$\frac{\partial U}{\partial A}$	Strain energy release rate
$p$	Stress intensity factor due to the external load
$f$	Tensile force due to the extension of FRP
$f_t$	Tensile strength of concrete
ole r nce	Tolerance
	Transpose
d	Total crack opening
$b_f, l_f$	Width and length of FRP
	Work done by externally applied loading
$c$	Young's modulus of concrete
	Young's modulus of FRP



## CHAPTER 1

### INTRODUCTION

#### 1.1 General

Sudden failure occurs in concrete structural members, such as beams and beam column joints, due to the quasi-brittle behavior of concrete. In general, sudden failures in concrete initiate with crack propagation in the tension zone because of an increase in stress or occurring initial crack. Therefore, these failures should be accurately predicted.

Material strength and fracture mechanics theories are two major groups of theories in crack growth analysis. In material strength theory, cracks are detected based on strain, stress, or a combination of stress and strain. The disadvantage of this theory is that eliminating the damaged elements can produce a stress singularity. Material strength theory, which existed prior to fracture mechanics, uses a crack propagation criterion with no strain energy effect. It explains the propagation of a crack as an unavoidable method of energy transfer between the strain energy of an elastic body and the fracture energy required to produce a new crack. The fracture mechanics theory is considered to be a more accurate method for predicting crack growth, because this method is similar to the physical reality of crack propagation (Shi, 2009). Accurate prediction of crack propagation in concrete is essential for improving its reliability, durability, and serviceability.

Two methods are now available for fracture analysis in concrete. These can be broadly categorized into Linear Elastic Fracture Mechanics (LEFM) and nonlinear elastic fracture mechanics.

LEFM was first used to study crack propagation in warships deployed during World War II (Esfahani, 2007). In this method, a coefficient is applied to the stress in the vicinity of the crack tip. This coefficient is called the stress intensity factor. Because the stress intensity factor depends on the material properties, size of the crack, load, and geometry of the structure, it presents a relationship between the material and the reaction of the structure. Stress singularity at crack-tip is the characteristic of Linear LEFM. Later, some studies used LEFM in crack propagation analysis, but Kaplan (1961) found that deploying LEFM was not acceptable when it came to solving crack problems with normal concrete. To solve the aforementioned problem, the first model based on nonlinear fracture mechanics for concrete was proposed by Hillerborg et al. (1976). Their study introduced a region, often termed fracture process zone (FPZ), which has the ability to transfer normal and shear stress to close the crack. The FPZ plays a remarkable role in the behavior of cracks and their propagation under load. Hence a study on the role of the FPZ to predict and prevent crack propagation under static monotonic load (Esfahani, 2007) is indispensable. Although more techniques of crack propagation have been developed in fracture

mechanics, crack modeling to predict the behavior of concrete structures is still far from satisfactory.

On the other hand, the crack patterns of concrete changes by using different types of reinforcement. Fiber reinforced polymer (FRP) composites have been increasingly used in concrete structures in recent years because of their corrosion resistance, low weight, high tensile strength and large strain. Nowadays, strengthening using FRP is of interest from an economic point of view. The use of FRP composites is now identified as a successful, suitable, and efficient technique to strengthen structures.

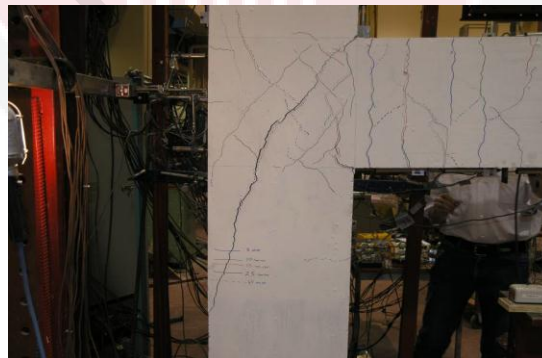
Because FRP modifies cracks propagation in concrete structural members, it is essential to study the crack behavior in structural members with FRP. Much effort has been made to model and test members flexural strengthened with FRP for different types of cracks, debonding, anchorage, and beam behavior (Arduini, et al., 1997; Yang et al., 2003; Bruno et al., 2007 ; Pavan et al., 2005; Achintha and Burgoyne, 2011). In addition, there have been many experimental studies on beams shear-strengthened using FRP (Adhikary and Mutsuyoshi, 2004; Coronado and Lopez, 2008; Teng and Hollaway, 2008; Siddiqui, 2009). Conventional theoretical and numerical analyses in fracture mechanics have been applied to study concrete flexural beams strengthened using FRP composites (Jae et al., 2010; Chen et al., 2012; Kesavan et al., 2013; Lin et al., 2014). However, there is still little in the way of theoretical analysis to calculate the fracture resistance of a shear-cracked beam strengthened with external FRP.

One of the most vulnerable structural elements to sudden failure is the beam column joint (Figure 1.1) whose crack propagation behavior needs to be predicted (Deaton, 2013). Cracks in the joint start where the beam and column intersect because of stress increase (Said and Nehdi, 2004). Thus, the fracture mechanics theory is the best method for predicting this type of crack. Beam column joints, which provide for the continuity of a structure, are a critical and significant part of concrete structures. Therefore, the study of their behavior, crack patterns, failure mode, and strengthening is essential. Presenting an accurate fracture mechanics model for beam column joints is of great importance owing to the complex behavior of the joints.



**Figure 1.1. Damage to Beam Column Joint (Said and Nehdi, 2004)**

Taylor (1974) was among the first to study joint behavior, and the American Concrete Institute (ACI) (1976) was the first to recommend codes for joint design. The behavior of a structural joint is very complex because of the interaction between different mechanisms. These different mechanisms are flexure, shear, and confinement of the joint. A crack may happen in the joint (shear failure) (Figure 1.2), in the beam, or in the column. Shear cracks in the joint are generally more significant than other types of cracks because shear failure in joints can break down the whole structure. These types of cracks have been observed in many studies (Pantelides et al., 2008). When a crack happens in a joint, the column and beam rotate and lose their load capacity. This mechanism is undesirable and it should be prevented. To prevent shear failure in the joint, it is better that the failure be shifted to an adjacent beam.



**Figure 1.2. Cracks in the Joint Tested (Corazao and Durrani, 1989)**

The strengthening of joints can be carried out using jacketing steel, bolted steel, or plate steel or by stitching and drilling with grout. Corrosion, confinement in the joint, and cost are reasons why these techniques are rarely used. Nowadays, to retrofit a beam column joint, external FRP is used. However, use of fracture mechanics modeling to analyze crack propagation in the joints and in joints strengthened by FRP has not been reported.

The prediction of crack propagation in concrete structures fails to conform to experimental tests.

Given these issues, the elastic stiffness of a fracture process zone is not applicable to the prediction of crack propagation in concrete. Thus, new fracture process zone stiffness and crack propagation criterion for concrete have been valuable to the study of concrete and have provided theoretical support to proposed empirical models.

The other contributions of this study have been summarized as follow:

- a) Development of a fracture mechanics model to calculate the dissipation of the release rate on beams with flexural and shear strengthened with FRP, which is essential for predicting crack propagation in concrete
- b) Proposed of a theoretical method to obtain the fracture resistance of shear-cracked concrete beams and beams shear strengthened using FRP
- c) Development of numerical model to predict crack propagation in beam column joints and FRP-strengthened joints

It is also believed that this investigation will impact professional practice. Also, the findings have informed the effect of FRP on crack propagation in concrete structures such as beams and joints and will contribute to the development of design guidelines.

## **1.2 Problem Statements**

Modeling of crack propagation is critical for simulation of failure in concrete structures. The common challenge for the modeling of crack propagation is the lack of a precise numerical solution for modeling material softening because the prediction of crack propagation in concrete structures fails to conform to experimental tests. In previous models, the stiffness of the FPZ was estimated from the Young's modulus (Gerstle and Xie, 1992; Elices et al., 2009; Sagaresan, 2012). However, the FPZ has softening behavior as well, which is not considered. Therefore, previous studies did not provide an accurate prediction of crack propagation. Hypothetically, in order to obtain accurate results, the softening effect of the FPZ needs to be considered when using the finite element method to model a material. Then, based on an accurate stiffness matrix, an improved Griffith energy approach (1921) can be used to predict the propagation criterion.

Recently, the application of FRP to strengthen concrete structures has significantly increased, and the stiffness and strength of concrete structural elements have been improved by applying FRP in the member. However, there is concern about crack propagation and crack patterns in concrete sections strengthened by FRP. There are many numerical models to predict cracks between FRP and concrete for various strengthened structures. However, most of them are based on stiffness and strength criteria. These numerical models cannot properly predict crack propagation in structural members retrofitted with FRP based on an energy approach.

It is essential to consider effect of FRPs on cracks propagation in concrete to prevent crack propagation. In previous models, the authors focused primarily on the occurrence of slip in FRP composites (Dai and Ng, 2014; Pan and Wu, 2014). Therefore, previous investigations did not take the influence of FRP on the FPZ and on crack propagation into account. Also, the effect of FRP on crack propagation based on an energy approach has not been investigated yet. It is a challenge that needs further study. This is what previous authors have done, and here is our contribution.

There are many theoretical models for calculating the loading capacity for various flexural–strengthened beams (Ohno et al., 2014 ). However, shear capacity of a beam shear strengthened with FRP composites has not been reported yet.

Many studies have been conducted on beam column joints with bonded FRP. However, most of these studies have been done experimentally; there is no research available on numerical modeling owing to the lack of proper finite element models of the fracture process in joints.

Cracks begin where columns and beams intersect owing to tension stress. Thus, fracture mechanics that takes into account tension softening is needed to analyze cracks in joints. Therefore, an accurate FPZ model of joints with and without FRP strengthening is required.

The aim of this study is to develop more accurate prediction of crack propagation in concrete structures particular reinforced concrete (RC) beams and RC beam column joints strengthened with FRP.

### **1.3 Objectives**

In order to achieve the above mention aims, the following objectives are outline:

1. To develop new formulation for FPZ stiffness and crack propagation criterion in RC beams and RC beams strengthened with FRP under static load.
2. To develop a new element and numerical model for the prediction of crack propagation in RC beams with FRP shear strengthened under static load.
3. To identify shear capacity theoretically for cracked RC beams with FRP shear– strengthened.
4. To implement the new developed FPZ stiffness and numerical model on beam column joints strengthened by FRP.
5. To validate the new proposed model by experimental tests on RC beam and RC beam column joints with and without flexural and shear FRP strengthening.



## **1.4 Scope and limitations**

The scope of the current research is as follows:

- a) This research focused on the FPZ stiffness and crack propagation criterion in concrete beams and the flexural and shears strengthening of concrete beams using FRP. It is also focused on the development of a numerical model for beam column joints strengthened by FRP under static load.
- b) Three beams were built and tested to validate the results obtained by the present model. All beams were tested under four-point loading with simple support. The beam B-0 was used as the control beam. Specimen B-1 was flexure-strengthened with CFRP sheets. The third specimen B-2 was strengthened in the shear span. Two full scale reinforced concrete beam column joints were tested to validate the results obtained by the present model. One beam column joint was tested in an un-strengthened condition to act as the control beam column joint. One beam column joint was strengthened with CFRP sheets.
- c) Analysis was carried out using fracture mechanics theory. In fracture mechanics, a crack is assumed to start in the tension face in concrete with strain-softening behavior. The stress-strain behavior of concrete in the compression zone and crushing are neglected in the fracture mechanics theory.
- d) Four-node isoparametric elements were used to model bulk concrete as a linear elastic material. The nonlinear behavior of bulk concrete and plastic deformation is ignored owing to the small deformation.
- e) Program code was developed for analysis of two-dimensional (2D) plane stress for fast convergence. Based on the literature review, when displacements are small, 2D modeling is sufficient for analyzing cracks (Gerstle and Xie, 1992). A three-dimensional (3D) model increases complexity, resulting in inaccuracy.
- f) An investigation of the effect of various material and geometric parameters (parameter study) was carried out and is discussed based on the numerical, experimental and theoretical results.
- g) Validation of the numerical model was carried out through numerical modeling by use of commercial finite element software and experimental tests for three types of reinforcement concrete beams with and without implementing FRP for flexural strengthening of beam under four-point loading. No internal stirrup was provided in the desired shear failure region. Also, two full-scale beam column joint specimens, one of which was strengthened using a CFRP sheet, were cast and tested under static load.

## **1.5 Layout of the Thesis**

The thesis is prepared in five chapters as follows:

Chapter 1 describes the importance of the current study, contribution of thesis, provides the problem statements, objectives, scopes and limitations.

Chapter 2 reviews earlier studies modeling crack propagation in concrete and previous efforts on the use of fracture mechanics to analyze FRP-strengthened concrete. This chapter also includes a literature review of previous efforts to analyze beam column joints strengthened using FRP composites.

Chapter 3 describes the proposed model of cracks in concrete in order to obtain accurate results under static load and the program code developed to model beams flexural strengthened and shear strengthened with FRP under static load. It also describes the experimental tests conducted on an RC beam and on concrete beams flexural strengthened and shear strengthened with FRP.

Chapter 3 also describes the development of the proposed theoretical method to estimate the fracture resistance of shear cracks in RC beams shear strengthened with FRP.

In addition, in Chapter 3 the developed numerical model is applied to predict crack propagation in beam column joints and in FRP-strengthened joints. The experimental tests on the beam column joints and the FRP-strengthened joints used to validate the present numerical model are also described.

Chapter 4 includes the program code used to estimate and validate the analytical and experimental results obtained from the current study, the results reported in previous studies, and the results obtained from the finite element analysis (FEA) software ABAQUS. Also, a number of beams were considered as benchmarks and their respective behaviors were observed. The effect of various material and geometric parameters (parameter study) was investigated and the results are discussed in this chapter. This chapter also validates and discusses a proposed theoretical model of a shear-cracked beam and provides theoretical analysis of the beam shear strengthened with an external FRP.

Chapter 4 also discusses the ability of the present nonlinear finite element code to estimate fracture mechanics and to model control beam column joints and joints strengthened with FRP. In this study, to validate the present model, the FEA software ABAQUS was used to model crack propagation by conventional cohesive elements (COH2D4P).

Chapter 5 provides conclusions from the present study with suggestions for future investigations.

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