



UNIVERSITI PUTRA MALAYSIA

***IMPLEMENTING NUMERICAL SIMULATION FOR STATIC AND
DYNAMIC LOADING OF CABLE-STAYED PENANG BRIDGE***

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**IMPLEMENTING NUMERICAL SIMULATION FOR STATIC AND
DYNAMIC LOADING OF CABLE-STAYED PENANG BRIDGE**

By

MOHAMMED IDRIS MOHAMMED

**Thesis Submitted to the School of Graduate Studies, Universiti Putra
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Doctor of Philosophy**

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DEDICATION

My Parents

My wife

My children

Brothers and relatives



Abstract of thesis presented to the Senate of Universiti Putra Malaysia in fulfillment of the requirement for the Degree of Doctor of Philosophy

IMPLEMENTING NUMERICAL SIMULATION FOR STATIC AND DYNAMIC LOADING OF CABLE-STAYED PENANG BRIDGE

By

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November 2016

Chairman : Associate Professor Faizal Mustapha, PhD
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Penang (I) bridge is classified as a Cable stayed bridge, which constructed globally for their long spans and aesthetics appealing. Factors such as high traffic volumes and geometrical movements cause increase in deflections and stresses on the bridge system, which at the end these defects possibly lead to degrade their service life cycle. Due to these reasons, Penang (I) bridge needs continuous monitoring i.e. a tool to resist any changes toward various loading conditions, such as sensors arrangement. Hence, the study aims to recommend sensors positions for Penang (I) bridge, by conducting modal analysis and simulation processes which introduced in the Finite Element Method (FEM). In addition, specific software called, 'Nastran & Patran' was used to compute the displacement, forces and stresses based on static symmetrical and unsymmetrical loading conditions, as well as mode shapes on dynamic loading condition. As a result, from these analyses, critical elements and their associated grids points of the bridge elements were identified, which eventually, sensors placement have been proposed. The results revealed there are 78 sensors can be placed at the entire structures, which are mainly located at the cables, towers and end side spans and main spans of the bridge. In term of contribution, the study noticed the complexity of Penang (I) bridge structures coupled with high traffic volumes might lead to damages at invisibles locations, which only can be identified using the finite element analysis. Therefore, the proposed sensors for the bridge probably can be used by the relevant authorities and perhaps as guidelines for future references.

Abstrak tesis yang dikemukakan kepada Senat Universiti Putra Malaysia sebagai memenuhi keperluan untuk Ijazah Doktor Falsafah

**MELAKSANAKAN SIMULASI BERANGKA MENGGUNAKAN STATIK
DAN DINAMIK UNTUK KABEL-PENAHAN JAMBATAN PULAU
PINANG (I)**

Oleh

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Jambatan Pulau Pinang (I), merupakan jambatan kabel-penahan yang dibina di peringkat global bagi jangka panjang dan mempunyai nilai estetika. Faktor-faktor seperti penambahan jumlah trafik dan peningkatan pergerakan geometri menyebabkan pesongan dan tekanan pada sistem jambatan, dan juga ia membawa kepada memendekkan kitaran hayat perkhidmatan mereka. Oleh kerana sebab-sebab ini, Jambatan Pulau Pinang (I) memerlukan pemantauan berterusan iaitu alat untuk menentang sebarang perubahan ke arah pelbagai keadaan beban muatan, seperti susunan sensor. Oleh itu, kajian ini bertujuan untuk mengesyorkan kedudukan sensor untuk jambatan Pulau Pinang (I), dengan menjalankan proses analisis dan simulasi modal yang diperkenalkan pada Kaedah Unsur Terhingga (FEM). Di samping itu, perisian yang dipanggil, 'Nastran & Patran' telah digunakan untuk mengira anjakan, daya dan tekanan berdasarkan keadaan bebanan simetri dan simetri statik, serta bentuk mod dengan bebanan dinamik. Hasil daripada analisis ini, unsur-unsur kritikal dan mata grid yang berkaitan dengan elemen jambatan telah dikenal pasti, yang akhirnya, penempatan sensor telah dicadangkan. Keputusan mendedahkan terdapat 76 sensor boleh diletakkan pada keseluruhan struktur, yang kebanyakannya terletak di kabel, menara dan rentang sampingan akhir dan rentang utama jambatan. Dari segi signifikan kajian, kajian mendapati struktur pembinaan Jambatan Pulau Pinang (I) yang agak kompleks dan ditambah pula dengan trafik yang tinggi mungkin membawa kepada kerosakan di lokasi yang sukar untuk dilihat, yang hanya boleh dikenal pasti dengan menggunakan analisis unsur terhingga (FEM). Oleh itu, sensor yang dicadangkan untuk jambatan mungkin boleh digunakan oleh pihak berkuasa yang berkaitan dan mungkin sebagai garis panduan untuk rujukan masa depan.

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I certify that a Thesis Examination Committee has met on 14 November 2016 to conduct the final examination of Mohammed Idris Mohammed on his thesis entitled "Implementing Numerical Simulation for Static and Dynamic Loading of Cable-Stayed Penang Bridge" in accordance with the Universities and University Colleges Act 1971 and the Constitution of the Universiti Putra Malaysia [P.U.(A) 106] 15 March 1998. The Committee recommends that the student be awarded the Doctor of Philosophy.

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LIST OF SYMBOLS AND ABBREVIATIONS

1-D	One-Dimensional
2-D	Two-Dimensional
3-D	Three-Dimensional
CBAR	Connected Bar
CBEAM	Connected Beam
CELASi	Elastic Springs
CMASSi	Concentrate Mass
CQUAD4	Quadrilateral Plate Element Connection
CROD	Connecting Rod
CSHEA	Shear Panel Element Connection
CTETRA	Four-Sided Element Connection
DEFORM	Specified Strain In Structural Element
DL	Dead Load
LL	Life Load
ML	Moving Load
EIGRL	Real Eigenvalue Extraction Data
FEM	Finite Element Method
FORCE	Concentrated Forces
FORCE1	Concentrated Forces
GRAV	Gravity force Format
HA	Normal Load
HA-KEL	Normal Load-Knife Edge Load
HA-UDL	Normal Load-Uniform Distributed Load
HB	Abnormal Load
MAT1	Isotropic Material
DL	Dead Load
ML	Moving Load
LL	Live Load
MOMENT1	Concentrated Moments
PBAR	Property Bar

PLOAD	Pressure Load
PLOAD1	Ditributed Load
PROD	Rod Properties
SHM	Structural Health Monitoring
SPC	Single Point Constraint
Sec.	section
E	Modulus Of Elasticity
G	Shear Modulus
m	Mass (Inertia)
k	System Stiffness
p	Applied Load Vector
u	Grid Point Displacements
\ddot{u}	Acceleration of Mass
\dot{u}	Velocity of Mass
ν	Poission's Ratio
ϕ	Eigenvector or Mode Shape
ω	Circular Natural Frequency
f	Natural Frequency

CHAPTER 1

INTRODUCTION

1.1 Research Background

Many countries have reflected substantial interest and / or concern toward their civil infrastructure, especially in cable stayed bridges. The bridges are elegant, efficient using material structure, excellent self-balancing structural system, superior overall stiffness, enhanced aerodynamic behavior, economical design and weight than suspension bridges. They comprise of principle components deck, one / more towers and Fan / Harp cables pattern. The incline cables are suspended from the high pylons and attached to the girder at grid points. The elasticity of stayed cables primarily grants the bridge longer spans without intermediate piers. Such that the dead load and live load effect at the girder-deck are transmitted to the towers through the cables (Wu and Wang, 2008; Lee, et al., 2008; Gimsing and Georgakis, 2011).

However, these large structures are experiencing inadequate and deterioration over its operating time. Hence, there is a necessity for continuous maintenance and rehabilitation scheduled services. In many engineering areas, monitoring techniques are employed to ensure the structure integrity and hence able to maintain its safety. The term, structural health monitoring actually deals with safety issue, which emphasizes on structural performance as well as contribute to saving cost of repairing. Hence, the application of continuous monitoring would delay or limit damage growth, as sufficient information of data related to damage rates exists (Zapico and Gonzalez, 2006). In addition, Deza (2004) stated bridges disposition demands for safety implication and clear economical so that the burden of repairing or replacing cost is refrained. Meanwhile, an accurate assessment of the present traffic load relevant to the bridge carrying capacity and anticipation of alteration in future loads or the structure capacity can cause deterioration in the applicable time span.

Despite experiencing deterioration over its operating time, the structures are also suffered from damage. It is described and characterized as the alteration in the designed components features of the infrastructure from geometry and material properties, which caused and involved unfavorable circumstance to structure integrity and its operative functionality (Farrar, et al., 2006). The damages of the bridge structure can be identified in two ways i.e. directly and/or indirectly. The first is identifying the damage type e.g. cracks due to corrosion / delamination, in which the appropriate inspection technique is used based on the physical phenomena and have local and direct application. While the latter is via global response characteristics of undamaged structure, where it compares the data before and after the damage. In both conditions, it requires reliable monitoring i.e. quantitative performance measures. The technique of monitoring previously was primarily based

on pessimistic prediction and periodic inspection which was referring to the direct approach (Staszewski, et al., 2004).

Furthermore, the bridges are also encountered the structural material failure, which is known as defect or flaw, due to improved loading condition. For examples the bridge may operate at its optimal level which resulted to corrosion and fatigue damages. While, environmental hazards such as floods or earthquake are the examples of scenario that caused bridge structure failure, where it inflicts the structure to the level of non-adequate to perform effectively (Farrar and Worden, 2007). Hence, in dealing with these issues, Civil Structural Health Monitoring (CSHM) has been introduced in implementing measurable periodical procedure to recognize the identity of damage strategy in the complex civil structures. These measures are analyzed to predict the structure physical condition (Farrar, et al., 2006).

Structural Monitoring covers modeling and analysis that are utilized to assess the integrity of the bridge in the sense of operating, ageing, and the level of safety. The implementation of structure health monitoring systems for bridges is to determine the vital logical explanations, which can be described in 6 levels; (1) the existence of damage in the bridge; (2) the location of this or these damages; (3) the character or particular kind of damage present; (4) the degree of damage and its extent level; (5) the consequence effect and the influence of the damage on the structure at present and future;(6) the bridge persistence situation to run for longer period than anticipated include forecasting its future behavior and integrity (Stubbs et al., 2000). In relation to the current study, it concerns on the first 2 levels of the implementation of structure health monitoring systems for bridge i.e. the existence and location of these damages.

1.2 Problem Statement

The global challenge of civil and mechanical engineering community is monitoring the performance of bridge structures, which are operating beyond than their design expectation. Factors such as aging, natural phenomena, incremental traffic volume and fluctuations functionality are detrimental causes which inflict and degrading bridge members operate during their life cycle (Frangopol, 2011). Significantly, the demand for high bridge performance and/or aging may result in the vulnerability of constructed facilities (Tsompanakis, 2010).

According to Cohen et al. (2007); Belluck, (2007); Doyle, (2009), previous statistics have indicated that around 41% of the USA'S 577,710 bridges are either suffers from structural deficient or functional obsolete. One of such cases is the total collapse of I-35W Highway Bridge of four lanes in the state Minnesota- Minneapolis city which use to carry 140,000 vehicles/day in 2007, the image of such collapsed is illustrated in Figure 1.1. Furthermore, the outcome of the investigation revealed that it was due to; (I) the migration toward opportunity in big cities leading to increase in traffic

volume; (II) the country economy declines; (III) lack of structural inspection which resulted in negligence of the corroded bridge members and become unsafe. Similarly, in China and India where bridges are essential to overcome river banks and/or passage overgrowing population area. Their bridges collapsed due aging, ineffective monitoring procedures, and overloading tracks. Meanwhile, in Queensland Australia alone, about 3000 bridges with an annual maintenance cost in excess of 20 million dollars and replacement value of two billion dollars (Tan et al., 2009).



Figure 1.1 : I-35W highway bridge collapsed in Minneapolis Aug. 2007
(Source: Salam and Helmy, 2014).

As bridges need to operate in any environmental conditions, their infrastructures are required to be sensed to perform their task for safety and ensure long term services. In order to understand their reaction, the involvement of embedded sensing system make them intelligently response to any suffering due to strain-displacement, temperature, internal forces, stresses and vibration that result in deterioration and /or deformation from their natural characteristic.

In relation to cables stayed bridges, many of them are still in use today and were designed according to the old traffic code. Presently, their infrastructures are experiencing to faster, heavier weight tracks and growing moving traffic volume than before. In order to continue using these structures, it is necessary to continue evaluate their loading-bearing capacity for pursuing of ensuring safe performance (Spyrakos, et al., 1999; Deza, 2004).

Without exception in Malaysia, Penang (I) Bridge is also classified as old cables stayed bridge. This bridge is located at the North-West of Peninsular Malaysia, it is about 13.7km long concrete civil structure, harp cable stayed and 4 lanes opened to traffic in September 1985 (McCabe et al., 1990). Due to increase in traffic capacity, the approach spans were winded from 2-lane to 3-lane dual carriageway. The expansion had included additional lane for motorcycle (King, et al., 2009). Also, Two-lane at the viaduct approach spans and two single-lane approach ramps were added (Buckby, et al., 2009; Corbett, et al., 2010). However, at present, the traffic

volume and the weight of vehicles are no longer the same as in the 1990's, these are due to the growth of industry and population at the mainland and Penang Island.

The highest traffic volumes recorded at rush hours between 6.00am to 8.00am, nearly thousand vehicles toward the island and similarly leaving the island at peak hour 4.00pm to 7.00pm¹. It is interesting to note that, even though Penang (II) Bridge was officially opened to traffic in the year 2014, but still Penang (I) Bridge dominates high percentage of the traffic volume. In addition, the result from the interview also revealed it was about 5% to 10% certain class of vehicles were reduced, since Penang (II) Bridge operates. A survey conducted by the Public Transport Department in 2010 found out that 70% of daily users of the bridge were from the mainland-Butterworth side going to the industrial zones of Penang Island which is closer to Penang (I) Bridge.

With regard to Penang (I) bridge stayed cables, the history indicated since 1996 to 1999 the bridge had been vulnerable to cables overstress in short cables M1 and E1². In 2000, new bearings positioned at the piers to limit the load on short cables. Simultaneously, two short cables coded M2 and E2³ were replaced. In 2003, the bridge installed with acoustic system for monitoring cables but still uncertainty for health the rest of the other cables. The delay in response to embedding monitoring instrument may be due to change of authority body.

In December 2004, cable coded M9 was substituted as a result of unexpected breakage. Then, later 80 couplers were replaced after being found defected. In another event, in 2006, southwest side of the bridge breakage of cable code E9 has been detected by the present monitoring system. Corrosion was common result to replace the cables by consulting companies from United Kingdom Atkin. Co (Hendy and Sandberg, 2004) and Systra Co. (Lam, et al., 2012). Figure 1.2 below shows the crack which located between cable anchorage and the concrete attachment to the bridge girder. Figure 1.3 depicts the corrosion induced in the cable coupler.

¹ Based on the survey interview with the PLUS officer on 13th March 2014

² Cable Codes for Penang (I) Bridge

³ Cable Codes for Penang (I) Bridge



Figure 1.2 : Structural defect of cable Anchorage with bridge girder
 (Source: HENDY and Sandberg-Atkins-2004)



Figure 1.3 : Illustrate the cable couplers before and after corrosion
 (Source: HENDY and Sandberg-Atkins-2004)

As far as Penang (I) Bridge is concerned, it also suffered from common breakage of cables, hence, the bridge has been installed with an additional system known as advitam “load cells”, which was placed at the cables. Its function is to determine loading magnitude and monitoring the capacity of cables, which at the end able to provide the data on attributing factors. The system revealed that Penang (I) Bridge is actually under deterioration as a result from trafficking loading and ageing factors. Despite cables, the other elements such as deck, pylon, piers, and piles are facing the same environmental conditions, which require for monitoring system to ensure its safety.

Based on the above discussion, there is a need of device to spot the damages. One of the solutions to detect damages is via sensors implementation. Sensors implementation globally is expected to detect damage and their reliability of the data collecting subject upon the sensor arrangements. Their average number relies upon the length of the infrastructure span. Additionally, limited sensor numbers and locations can constraint the measured data for only certain regions i.e. poor measurement performance (Ismail, 2011).

Traditionally, the global sensing techniques are more effective to assess the entire bridge performance. To implement global sensing device of various types, it requires identifying the hot spot stresses regions to position these sensors. Thus, in order to assist analysis alteration in properties and predict damages easily, Penang (I) Bridge needs to have quantitative global sensors placement for overall structure system, in addition to the existing local sensor locations. This is also supported by the result of the interview conducted earlier with PLUS officer, where it revealed that the bridge has minor defects at certain locations. Due to the issues highlighted above, the study is conducted based on several objectives.

1.3 Research Objectives (RO)

The aim of the research is to employ finite element analysis of Penang (I) Bridge to monitor the structure and install sensors. The bridge is assessed with real-time live traffic load effect conditions. The objectives of the study are explained in the following sequences:

- i. To perform static analysis of symmetrical dead load case, additional live load case according to British Standard BS 5400 and identify bridge critical points for sensor placement.
- ii. To conduct unsymmetrical analysis at the combine dead and live loads and investigate the bridge critical points for sensor placement.
- iii. To carry out dynamic analysis at dead load stage and determine frequencies with relevant mode shapes.

1.4 Scope of Research Work

The present study focuses on evaluating the Penang (I) bridge structure using analytical modal data as to determine the weakness of its members. Factors such as location, size and cost of construction demanding the bridge to be assessed and it is also important to note that the structure itself is nearly 30years in service. Due to these facts, the bridge might be experience deterioration, damage, structural material failure and others. Therefore, the bridge needs a proper continuous monitoring tool i.e. sensors in detecting the failure of its members.

Hence, realizing the importance of having sensors arrangement for Penang (I) Bridge is the main goal of the research. Therefore, the current study applied nonintrusive approach by simulating the bridge model as proposed by (Ragland et al., 2011). In terms of modal analysis and simulation, in line with (Miao, et al., 2007; Li, et al., 2010), this research focusing on linear ‘symmetrical and unsymmetrical’ method of finite element and the logical outcome based of static and dynamic responses were used to propose locations of the sensors.

As for the analysis part, the finite element software program i.e. Nastran and Patran program has been selected in the current study, which was used by National Aeronautics and Space Administration (NASA) for general finite element solution instead of SAP2000⁴ which is useful for elastic second order cable element response (Thai and Kim, 2008).

This is vital to identify the locations of highly stressed members i.e. its critical grid points, which eventually lead to sensors placements in the overall bridge infrastructure. In relation to this, the study considered the loading condition in the analysis. It used the standard to develop the symmetrical and unsymmetrical analysis which at the end, the results would indicate the proposed sensor locations.

The details of the research procedure are summarized and pursued as follows:

- To determine the objects and investigate the structure problem and to exploit the scope of the research.
- To review previous scholars works on bridge monitoring using finite element and identify their limitations.
- To employ modal analysis and simulation by adopting a static approach of dead load and to determine the critical areas of the bridge members.
- To include British Standards BS5400 as live traffic load to the infrastructure and evaluate the bridge elements for crucial spot of stresses.
- To establish unsymmetrical condition due to the previous combined static loads and assess the bridge behavior.
- To apply dynamic analysis at the dead load case and to determine the critical areas of the bridge members.
- To discuss the result and findings of analysis in each case of the analysis.
- To propose sensor locations at each step of the analysis to all bridge elements.
- To conclude and state objective achievement.

1.5 Thesis Organization

Basically, the thesis is orderly split up into five chapters in the following manner; Chapter 1: begins its journey by addressing the background of the structural health monitoring to be proposed on bridge civil structure. It covers the background of the research, problem statement that triggers the interest of the study. Then it continues with the objectives of the research, scope of the research work and the importance of the investigation to Structural Monitoring application which is related to maintenance and rehabilitation activities for instance frequent visual inspection, repair, sensing monitor and managing bridge structures.

⁴ Finite element software

Chapter 2: Past literatures on related topics have been covered at this chapter. Basically, it provides background the bridge structures, bridge monitoring, the concept of finite element concept which explains the static and dynamic analysis, bridge loading aspect as well as the sensor arrangements. Finally, the gap is highlighted at the end of the discussion.

Chapter 3: portrays the way the study is conducted. The use of finite element method and its contribution in structural analysis is explained. The model of Penang (I) Bridge is created with the aid of NASTRAN and PATRAN program.

Chapter 4: presents the modal analysis and simulation results and discussion. The discussion of the results is divided into 2 main parts i.e. a) Static loading analysis and b) dynamic loading analysis. For Static loading analysis, it further divided into 2 parts, which are symmetrical and unsymmetrical analysis. The discussion starts with symmetrical static loading aspect, where the results is sub-divided into 2 cases, a) static analysis of symmetrical dead load case and b) static analysis of symmetrical live traffic load case using British Standard 'BS 5400'. Later, for unsymmetrical Static Loading aspect, both scenario i.e. the dead load and live load case are considered during the analysis. While, Dynamic analysis is performed to derive the frequencies and mode shapes of the bridge at the dead load case. All the analysis is conducted in order to determine the critical grid points, which eventually lead to proposed location of sensors placements.

Chapter 5: Conclusion and recommendation for future works, this final chapter reveals prediction and predetermined summary of the three investigated objectives. Finally, it highlights the conclusion as a result from the findings, which include recommendations for future research.

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