



UNIVERSITI PUTRA MALAYSIA

***FLEXURAL RESPONSE OF REINFORCED CONCRETE BEAMS WITH
EMBEDDED CFRP PLATES***

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**FLEXURAL RESPONSE OF REINFORCED CONCRETE BEAMS WITH
EMBEDDED CFRP PLATES**

By

RACHAEL BUKOLA OHU

**Thesis Submitted to the School of Graduate Studies, Universiti Putra Malaysia,
in Fulfilment of the Requirements for the Degree of Doctor of Philosophy**

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DEDICATION



'TO JEHOVAH ALMIGHTY'

AND

TO MY PRECIOUS FAMILY

Abstract of thesis presented to the Senate of Universiti Putra Malaysia in fulfilment of the requirement for the degree of Doctor of Philosophy

**FLEXURAL RESPONSE OF REINFORCED CONCRETE BEAMS WITH
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November 2012

Chairman : Professor Mohd Saleh Jaafar, PhD

Faculty : Engineering

Fiber reinforced polymers (FRP) are used as either internal or as external reinforcements in structures. However, issues related to reduced ductility performance and large deflections have been observed by several other researchers which needs to be addressed in order for these reinforcements to be more widely adopted as alternative structural reinforcements in practice.

In this regard, an alternative method of using FRP plates as internal reinforcements in concrete beams is explored and presented in this thesis in terms of ductility/deformability performance as well as other structural responses under static loading with the aim of improving the ductility/deformability response as well as examining some aspects of structural behaviour. In addition, the bond behaviour of this reinforcement which is a key factor towards the improvement of structural performance especially with regards to the bond-slip behaviour at service and ultimate conditions was

studied. Thus, different surface treatments have been experimentally investigated through pullout tests to identify the best bond effects. Based on the experimental bond-slip behaviour obtained, a concrete – CFRP plate bond interface model is proposed and incorporated into a finite element algorithm for the analysis of concrete beams reinforced with embedded CFRP plates taking into consideration differences in the surface textures of the embedded CFRP plate. A 2-D nonlinear finite element program was thus adopted for the analysis of the proposed reinforcement technique. The most suitable surface treatment obtained from the pullout tests was then adopted in the embedded carbon fiber reinforced polymer plates (CFRP) in concrete beams via experimental testing under flexural load.

The results showed that embedded CFRP plates in concrete beams is an effective alternative form of reinforcement with a 37% decrease in deflection response and a 54% improvement in deformation/ductility performance. In addition the bond behaviour is dependent on the type of surface treatment with an increase in bond strength ranging between 78% - 284%. While an increase in concrete strength led to a 58% increase in the bond strength of embedded CFRP plates in concrete. Similarly, the proposed bond model for the embedded CFRP plates successfully depicted the concrete-CFRP plate interface behaviour and the FE results were in agreement with the experimental results with a percentage difference of 12% exhibiting a realistic simulation of the experimental load-deflection response.

Abstrak tesis yang dikemukakan kepada Senat Universiti Putra Malaysia sebagai memenuhi keperluan untuk ijazah Doktor Falsafah

**TINDAKBALAS LENTURAN BAGI RASUK KONKRIT BERTETULANG
DENGAN PLAT CFRP TERBENAM**

Oleh

RACHAEL BUKOLA OHU

November 2012

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Polimer Bertetulang Gentian (FRP) biasanya digunakan sebagai tetulang dalaman atau luaran di dalam struktur. Walaubagaimanapun, isu berkaitan dengan pengurangan sifat kemuluran dan jumlah lenturan yang besar perlu ditangani berdasarkan pemerhatian beberapa pengkaji bagi membolehkan gentian bertetulang ini digunakan dengan lebih meluas sebagai tetulang alternatif dalam struktur yang sebenar.

Sehubungan dengan ini, kaedah alternatif menggunakan plat FRP sebagai tetulangan dalaman bagi rasuk konkrit telah diterokai dan dipersembahkan dalam tesis ini dari segi keupayaan kemuluran/ubahbentuk dan juga lain-lain tindakbalas struktur apabila beban statik dikenakan dengan matlamat untuk meningkatkan tindakbalas kemuluran/ubahbentuk juga memeriksa beberapa aspek perlakuan struktur. Disamping itu, perlakuan ikatan bagi tetulang ini yang merupakan faktor utama kearah menambahbaik keupayaan struktur terutamanya berkaitan dengan perlakuan ikatan-

gelinciran pada keadaan khidmat dan muktamat juga dikaji. Oleh sebab itu, penggunaan rawatan permukaan yang berbeza telah dikaji secara ujikaji melalui ujian tarik keluar untuk mengenalpasti kesan ikatan yang terbaik. Berdasarkan perlakuan ikatan-gelinciran yang diperolehi dari ujikaji, model ikatan permukaan konkrit- plat CFRP dicadangkan dan digunakan didalam algoritma unsur terhingga bagi analisis rasuk konkrit bertetulang plat CFRP terbenam dengan mengambilkira perbezaan tekstur permukaan plat CFRP terbenam. Progam 2D unsur terhingga tak lurus telah digunakan untuk menganalisis teknik tetulang yang dicadangkan. Rawatan permukaan yang paling sesuai berdasarkan ujikaji tarik keluar telah diadaptasi dalam rasuk konkrit bertetulang plat CFRP terbenam melalui ujikaji beban lenturan.

Keputusan ujikaji menunjukan plat CFRP yang dibenamkan dalam rasuk konkrit adalah satu kaedah alternatif tetulang yang efektif dengan 37% pengurangan tindakbalas lenturan dan 54% penambahbaikan perlakuan kemuluran/keupayaan ubahbentuk. Disamping itu, perlakuan ikatan adalah bergantung kepada jenis rawatan permukaan yang menunjukan peningkatan kekuatan ikatan dari 78% - 284%. Juga, peningkatan kekuatan konkrit membolehkan 58% peningkatan kekutan ikatan plat CGRP terbenam dalam konkrit. Dalam masa yang sama, model ikatan yang dicadangkan bagi plat CFRP terbenam telah berjaya menggambarkan perlakuan permukaan antara konkrit- plat CFRP dan keputusan unsur terhingga adalah selari dengan keputusan ujikaji dengan peratsu perbezaan sebanyak 12% menggambarkan simulasi yang realistik bagi tindakbalas ujikaji beban- lenturan.

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I certify that a Thesis Examination Committee has met on the 7th of November 2012 to conduct the final examination of Rachael Bukola Ohu on her thesis entitled " Flexural Response of Reinforced Concrete Beams with Embedded CFRP plates " in accordance with the Universities and University Colleges Act 1971 and the Constitution of the Universiti Putra Malaysia [P.U.(A) 106] 15 March 1998. The Committee recommends that the student be awarded Doctor of Philosophy.

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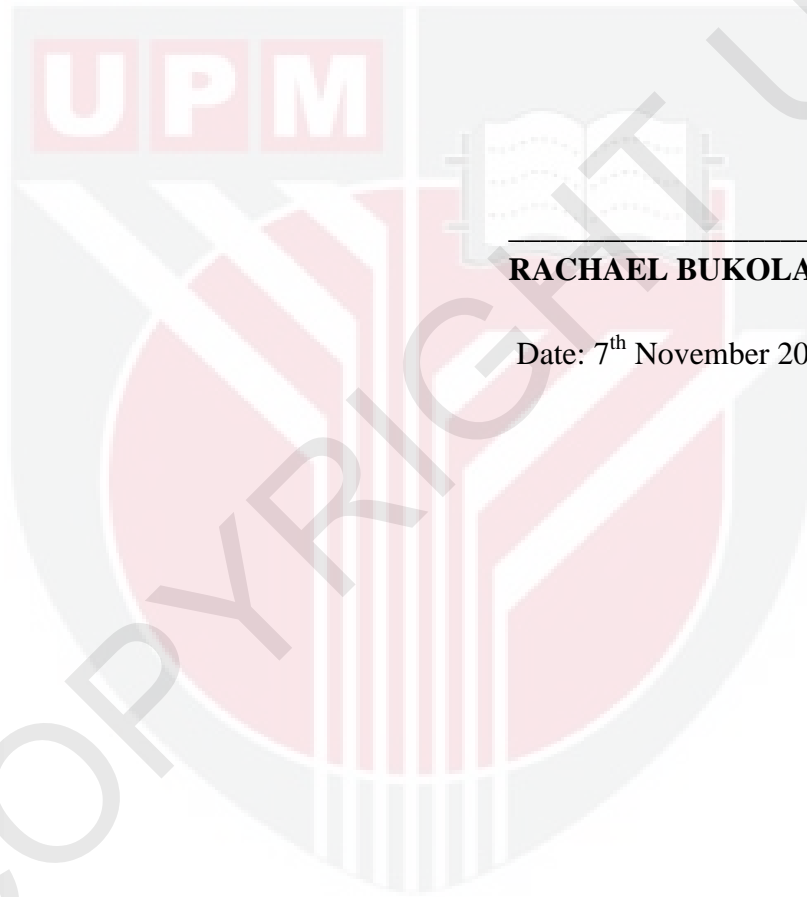
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DECLARATION

I declare that the thesis is my original work, except for quotations and citations, which have been duly acknowledged. I also declare that it has not been previously, and is not concurrently, submitted for any other degree at Universiti Putra Malaysia or at any other institution.



RACHAEL BUKOLA OHU

Date: 7th November 2012

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LIST OF ABBREVIATIONS

a = depth of equivalent rectangular stress block

A_f = FRP reinforcement area

A_s = area of steel and embedded CFRP plates

b and d = concrete section width and depth to reinforcement from compression fibre

b = width of beam (mm)

β_1 = factor taken as 0.85 for concrete strength f'_c up to and including 28MPa

$[B]$ = the strain-displacement matrix

$[B]_j$ = the strain displacement matrix of the interface element

d = diameter

D^2 = depth of beam (mm)

$[D]$ = global elasticity matrix of the element material

E_f = design or guaranteed modulus of elasticity of FRP

ε_{cu} = ultimate strain in concrete

f'_c = compressive strength of concrete

f_f = stress in FRP reinforcement in tension

f_s and f_f = the steel strength

f'_t = Split tensile strength (N/mm²)

f_{fu} = design tensile strength of FRP, considering reductions for service environment

F_r = modulus of rupture (MPa)

$[J]$ = Jacobian matrix

$[K]$ = interface element stiffness matrix in the global direction

l = cylinder length

l_b = embedment/bond length

L = Span length

M_n = nominal moment capacity

M_u = maximum ultimate bending moment (kNm)

n = number of sampling points

p = constant material parameter

P_{max} = maximum load

ρ_f = FRP reinforcement ratio

ρ_b or ρ_{fb} = FRP reinforcement ratio producing balanced strain conditions

ρ_{max} = maximum allowable reinforcement ratio

ρ_{min} = minimum allowable reinforcement ratio

s_s = shear slip (mm)

t = thickness of the element in the direction normal to the plane

τ = maximum shear bond stress

τ_o = interface shear bond strength

τ_u = shear bond strength (MPa)

τ_{bmax} = interfacial bond strength

T = splitting tensile strength

μ = interface coefficient of friction

w_f and t_f = plate/strip width and thickness

w_i and w_j = weighting coefficients appropriate to the position i and j

w_{max} = maximum allowable reinforcement index

Δu_i and Δv_i = relative displacements of the nodal points at the middle surface (a , b and

c)

ε , σ = instantaneous values of the stress and strain

ε_o , σ_o = the ultimate peak strain and stress

ε'_s ε'_n = tangential and normal strains at a point (shear slip and normal displacement)

σ_n is the normal stress (N/mm²)

σ_t = tensile bond strength on the interface (N/mm²)

$\{\delta\}$ = vector of the displacements (u , v)

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CHAPTER 1

INTRODUCTION

1.1 General

For a long time, steel has been the dominant type of reinforcement for concrete structures. Its compatibility with concrete and strength has made it an efficient reinforcement. However, deterioration and even collapse of steel reinforced structures has shown the sensitivity of steel in terms of its corrosive activity. Corrosion affects several aspects of structural behaviour with the most fundamental being bond. The transfer of stresses between concrete and reinforcement at both serviceability and at the ultimate state are considered to rely strongly on the quality of bond while mechanisms that resist flexural bending, shear and torsion are connected to the development of adequate bond characteristics. When corrosion takes place, it results in a degradation of bond and the ultimate bond strength is reduced (Almusallam et al. 1996) or it could ultimately lead to structural failure (Johnson, 2010). A picture showing corrosion in reinforced concrete and its effect on bond is shown in Figure 1.1 where it can be seen that high levels of corrosion result in significant reductions in bond strength due to losses of confinement and adhesion strength as well as reductions in geometrical characteristics (Bhargava et al. 2007). Because corrosion can not be totally eliminated researchers looked for other types of materials that could be used as alternative reinforcements. This led to the introduction of composite materials. Apart from this detrimental factor, others such as high maintenance costs, durability concerns and limited service life of traditional

structural materials can be made up for with the substitution of composites. Although composite materials have existed for a long time it took a while for it to be recognized as a type of reinforcement in civil engineering.

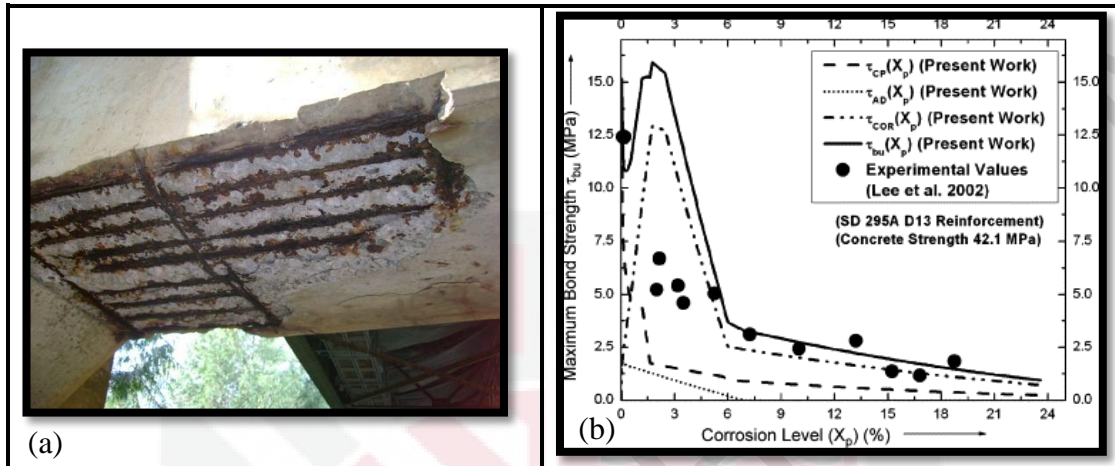


Figure 1.1 (a) Beam corrosion; (b) Bond strength as a function of corrosion (Lee et al., 2002 and Bhargava et al. 2007).

1.2 Versatility of Fiber reinforced polymers in construction

The use of composite materials in various aspects of civil engineering has grown steadily over the years. Just as material technology has evolved, so have structural techniques. Because of this evolution, several codes of practice have been modified to include and to cater for recent developments in research. To this end, fibre reinforced polymer composites have become one of the most versatile materials in use today. From being used for boat hulls, marine vessels, in the automotive industry and in aeronautics to being used as external or internal reinforcements of concrete structures. The versatility of FRPs stems from its' inherent properties some of which include its light weight, corrosion resistance and ease of use. Fiber reinforced polymers are currently manufactured in a variety of forms, shapes and textures. This

variety also extends to its properties however these properties differ from those of steel. Depending on the structural requirement, engineers can chose what is best suitable for a particular structure and this has helped to increase the popularity of this composite material worldwide. As of today, FRP bars are used as either internal or external reinforcements in the construction industry for reinforced concrete structures while FRP plates or strips are traditionally used as external reinforcements. Some of the current applications of FRP bars used as internal reinforcements include bridge decks, multi-storey buildings, parking structures and industrial buildings to name a few. In order to further increase the popularity of using FRPs as well as its diversity of utilization in reinforced concrete structures certain aspects of FRP behaviour needs to be further understood and improved on which will instil more confidence in it's use as an innovative composite material by engineers.

1.2.1 FRP – reinforced concrete behaviour

Past research has shown that externally bonded FRP reinforcements (EBR) or near surface mounted FRP reinforced (NSM) structures which are the most widely used techniques show increased capacity resistance (Capozucca et al. 2002), improved cracking and stiffness (Li et al. 2006), significantly enhanced load carrying capacities in bending (Barros and Fortes, 2005) and an increase in the equivalent longitudinal reinforcement (laminates) resulted in improved overall structural performances (Barros et al. 2007 and Yost et al. 2007). Some characteristic behaviour of FRP bars in concrete have shown that larger deflections and crack widths occur in FRP reinforced structures than in steel reinforced structures (Bakis et al. 2002). In addition, the FRP beam reinforcement ratio did not affect the moment capacity

(Nawy and Neuwerth, 1971) and the development of its full strength when used as reinforcement could not be achieved. Apart from these characteristics another common feature of FRP reinforced concrete was that of debonding and reduced ductility (Liu et al. 2006, Kang et al. 2005 and Li et al. 2008). Serviceability concerns of FRP-concrete behaviour has also been highlighted by Arockiasamy, 2000; El-Salakway et al., 2004, Salib and Abdel-Sayed, 2004, Saika et al., 2007 where the general agreement is that the low elastic modulus of FRPs leads to a lower serviceability performance in comparison to its steel reinforced counterparts.

Another aspect of FRP to concrete behaviour that affects its usage in the construction industry is its ductility performance. Ductility is a concept which allows a dissipation of large amounts of energy in a system which in turn serves as a means of giving warning signals of impending failure and is thus a measure of safety. The non-yielding nature of FRPs resulting from its linear elastic response to failure makes the ductility of FRP systems quite small in contrast to steel reinforced systems however because FRP systems do undergo sufficient deformation with an associated dissipation of energy, some researchers prefer to adopt the term 'deformability' (Jaeger et al. 1995, Vijay et al. 1996 and Newhook et al. 2002) in place of ductility for FRP reinforced systems. Based on this concept several researchers found out that FRP reinforced member's exhibit sufficient deformability (Vijay et al. 1996; Mufti et al. 1996; Shin et al. 2009 and Issa et al. 2011) and that the preferred concept of over-reinforced design adopted for FRP systems led to a more gradual failure with adequate warning.

Of all these aspects of behaviour, one of the most essential properties in an RC section for it to achieve an adequate level of performance is bond. Through bond a transfer of forces or stresses between the concrete and reinforcement occurs which allows a concrete section to develop strength and work as a single unit to resist external forces or loads. Three kinds of bond mechanisms typically occur; chemical adhesion, friction and mechanical interlock however, according to Cosenza et al. 1997; Wang et al. 1999; Benmokrane et al. 2002 and Firas et al. 2011, three (3) main factors control bond between FRP and concrete;

1. Chemical bond
2. Friction due to surface roughness of the bars
3. Mechanical interlock

Due to large differences in surface deformations, configurations or textures of FRP bars, differences in the bond strength achieved also differs. However the general consensus is that the bond strength of FRPs is lower than that of steel bars. Improving the bond strength can however be achieved by applying different surface treatments which could lead to bond strengths that are twice that of steel bars (Al-mahmoud et al. 2007; Makitani et al. 1993; Rosetti et al. 1995). Detailed investigations into the effects of elastic modulus (Achillides, 1997; Tepfers et al., 1997), effects of surface texture (Itoh et al., 1989; Makitani et al., 1993; Hattori et al., 1995; Jerrett and Ahmad, 1995), effects of cross sectional shape (Achillides et al., 1997) and effects of concrete strength on the bond of FRP bars has been carried out in several studies. Of these, the most popular effect considered is that of surface

texture while others like Lee et al. (2008), Achillides (1998) and Okelo and Yuan (2005) studied the effect of concrete strength.

1.3 Problem Statement

Based on the above, the following problems have been identified;

A Reduced ductility performance of FRP reinforced concrete beams was observed in comparison to that of steel reinforced concrete beams mainly due to the linear elastic behaviour of FRPs up to failure. Secondly, larger deflections and crack widths at service were shown to occur largely due to the lower elastic modulus of FRPs. In addition, there was an inefficient utilization of the full tensile strength of FRP reinforcements which is necessary to gain the full benefits arising from the high tensile strength property of this composite material. Therefore as a means of providing solutions to these problems alternative techniques utilizing FRPs need to be examined.

1.4 Objectives

The main objective of this study is;

1. To determine the effectiveness of embedded fibre reinforced polymer plates as longitudinal reinforcements in concrete beams under bending.

In addition the following are sub-objectives of this study;

- i. To evaluate the ductility performance and serviceability behaviour of concrete beams reinforced with longitudinal embedded fibre reinforced polymer plates.

- ii. To ascertain the bond behaviour and characteristics of embedded fibre reinforced polymer strips in different concrete compressive strengths.
- iii. To propose a bond model that represents the FRP-concrete interface behaviour using a 2D nonlinear finite element program.

1.5 Scope of the study

The scope of this study includes the experimental investigation on the effects of embedded CFRP plates of (tensile strength = 2800N/mm^2 and modulus of elasticity = $169,000\text{N/mm}^2$). The bond behaviour involved the use of various types of surface treatments and concrete strengths ($f_{cu} = 43.52\text{MPa}$ and 93.40MPa) in addition to the study of the structural response of this type of beams. The experimental bond tests were carried out via the use of standard sizes of $150\text{Ø} \times 300\text{mm}$ cylinders while the experimental testing of 21 model scale beams of size $150 \times 150 \times 750\text{ mm}$ and 18 large scale beams of size $160 \times 250 \times 2500\text{mm}$ reinforced with steel bars or embedded CFRP plates and external/near surface mounted CFRP reinforcements was carried out. Although two (2) different concrete strengths were adopted for the bond tests, only one (1) concrete grade was employed for the beam tests.

Determination of the characteristics of bond behavior was made with respect to the effects of various surface treatments and different concrete strengths. In terms of the structural behavior of these beams, only some critical aspects of flexural behavior were examined which include; load-deflection response, deformation/ductility performance, cracking characteristics, ultimate loads and associated failure mechanisms. The numerical study was carried out using a 2D finite element program

that was modified based on the experimental study. A bond model was proposed based on the experimental bond-slip relationship obtained which was then incorporated in the program for the simulation.

1.6 Research Significance

The possible application of fibre reinforced polymer plates as a replacement for longitudinal steel reinforcements or as an alternative to FRP bars would result in; (1) reducing the quantity of reinforcement used through an efficient maximization of the higher surface to cross sectional-area ratio; (2) an improved capacity performance due to larger ultimate strain capacity over that of FRP bars; (3) reduced long term maintenance or life cycle costs and (4) it could also be a useful alternative for section enlargement of structural elements without the need to significantly increase concrete member size.

The benefits mentioned in (1) and (2) in addition to increased flexural stiffness and high tensile strength rigidity of the CFRP plate would thus lead to a reduction in deflection and an increase in the ductility/deformability performance.

1.7 Limitations

Limitations of this study in terms of the issue of batching the concrete requires careful implementation. The process of batching these beams in practice needs to be carefully carried out and monitored to ensure that minimal or no voids are created or left at the soffit of the beam after casting due to the width of the plate. The use of properly sized concrete biscuits (casted in advance prior to casting) is essential to

maintain the concrete cover throughout the beam length in addition, proper concrete vibration using a small vibrating nozzle to ensure it can be easily maneuvered at all the edges of the mould to allow the proper flow of fresh concrete under the embedded CFRP plate. In practice, careful monitoring of the casting process is essential and needs to be carried out in batches, however because of the relative thinness of the embedded CFRP plate and as long as the workability of the fresh concrete is ensured, a good outcome of the structural element can be achieved. In addition, aggregate sizes not exceeding (that is $<$) 10mm need to be adopted when casting concrete where embedded FRP plates are to be used. As long as these measures are adhered to, an acceptable concrete structure that meets required standard practices can be obtained.

Some variations in the F.E cracking results can also be minimized further by improvements in the solution algorithm procedure and finite element mesh which affect F.E nonlinear analyses.

1.8 Summary

This chapter gives a brief description about the advent and usage of fiber reinforced polymers (FRPs) in concrete. The general characteristic behaviours of FRPs when used as either external or internal reinforcements has been highlighted in addition to the associated problems observed when FRPs are used. As a means of solving these problems, objectives of this study have been chosen with the aim of finding possible answers to these problems and hence provide an alternative technique of reinforcing concrete that is structurally beneficial, while also investigating the characteristics of this alternative reinforcing technique.

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