

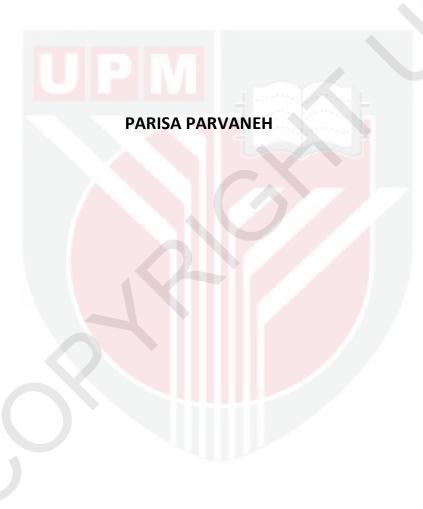
UNIVERSITI PUTRA MALAYSIA

GIS-BASED MODELING OF THE CHANGES IN WATER LEVEL DUE TO FLOOD WALLS

PARISA PARVANEH

FK 2010 92

GIS-BASED MODELING OF THE CHANGES IN WATER LEVEL DUE TO FLOOD WALLS



Master of Science Universiti Putra Malaysia,

GIS-BASED MODELING OF THE CHANGES IN WATER LEVEL DUE TO FLOOD WALLS

Ву

PARISA PARVANEH

Thesis Submitted to the School of Graduate Studies, Universiti Putra Malaysia, in Fulfilment of the Requirements for the Master of Science

August 2010

DEDICATION

Dedicated

To my beloved parellts Jor their help alld support.

To my husbandJor his love and patience.



Abstract of thesis presented to the Senate of Universiti Putra Malaysia in fulfilment of the requirement for the degree of Master of Science

GIS-BASED MODELING OF THE CHANGES IN WATER LEVEL DUE TO FLOOD WALLS

By

PARISA PARVANEH

August 2010

Chair: Associate Professor Abdul Ralim Ghazali, PhD

Faculty: Engineering

Flood control structures may protect an area against flood; however, it may also

induce inundation in another area at the same time. One of such structural projects is

constructing flood walls along a river reach. This study investigated the effects of

these flood walls on the upstream of the protected river reach using a GIS-based

modelling approach. The framework developed in this study covered the problems

encountered in the implementation of the GIS-based hydraulic models in the regions

with insufficient integrated data. The study area covered the reach of Karoon River

which is located between the Ahwaz and Farsiat hydrometric stations in Khuzestan

Province, Iran. The area was selected primarily due to the current and future

development of both the residential and industrial centres along this reach, as well as

because of the availability of data for the study area. Ahwaz City, which is located at

the upstream of this reach, is frequently subjected to flood and flood-related

problems. The flood walls on both river banks at the selected reach were simulated

in order to compute any changes in the water level and the flow velocity at the

111

Ahwaz hydrometric station. This simulation approach integrated both the ArcGIS tools and HEC-RAS hydraulic model by interfacing the HEC-GeoRAS extension. The river reach and the required features of the HEC-RAS model were digitized and extracted from Triangular Irregular Network (TIN) in the ArcGIS. Meanwhile, the HEC-RAS model was applied for the existing condition with no flood walls and the model was calibrated with the observed data. The restriction on the widths of river cross sections was carried out for a length of I km using three different methods to represent the flood walls. Based on the findings of the study, the best method was subsequently selected, and this was to use flood walls for the whole reach. Flood walls were added to the model using two different designs. In the first design, the widths of the cross sections along the equal length of the river banks (10 km) were confined by the flood walls at three different distances from the Ahwaz station so as to investigate the effects of these distances. The second design incorporated 30 km continuous flood walls, beginning from 10 km downstream of the Ahwaz station. Changes in the water level and the flow velocity at the Ahwaz station, due to the different lengths of the flood walls and the various distances, were determined and analyzed for seven return periods. The increases in the water level for the first design were found to vary from 0.66 m to 1.44 m, and it reached 2.32 m for 100 years return period in the second design. The resulted charts can aid engineers to make judgments on such flood protection techniques. The framework developed in this study could be used as a prototype simulation method for other rivers and to be implemented for different lengths of flood walls at any distance from any upstream gauge.



Abstrak tesis yang dikemukakan kepada Senat Universiti Putra Malaysia sebagai memenuhi keperluan untuk ijazah Master Sains

PEMODELAN BAGI PERUBAHAN ARAS AIR YANG DISEBABKAN OLEH DINDING BANJIR BERDASARKAN GIS

Oleh

PARISA PARVANEH

Ogos 2010

Pengerusi: Profesor Madya Abdul Halim Ghazali, PhD

Fakulti: Kejuruteraan

Struktur tebatan banjir mungkin melindungi sesuatu kawasan daripada banjir, namun pada masa yang sarna ia boleh mengundang banjir di kawasan lain, yang perlu dikaji. Salah satu daripada projek struktur jalah membina dinding banjir di sepanjang satu jangkauan sungai. Dalam kajian ini kesan dinding banjir ke atas kawasan di hulu kawasan yang dilindungi dikaji melalui pendekatan pemodelan berdasarkan GIS. Rangka kerja yang dibangunkan dalam kajian ini meliputi kaedah menggunakan model hidraulik berdasarkan GIS dalam kawasan yang tidak mempunyai data yang lengkap. Kawasan kajian ini adalah satu jangkauan Sungai Karoon yang terletak di antara stesen hidrometrik Ahwaz dan Farsiat dalam daerah Khuzestan, Iran, yang dipilih kerana pembangunan pusat penempatan dan industri di sepanjang jangkauan ini. Bandar Ahwaz, yang terletak di hulujangkauan ini, kerap dilanda masalah banjir. Dinding banjir pada kedua-dua tebing sungai yang dipilih ini disimulasikan bagi menentukan perubahan dalam aras air dan halaju aliran di stesen hidrometrik Ahwaz. Pendekatan simulasi ini menggabungkan alatan ArcGIS dan model hidraulik HEC-RAS dengan mengantaramuka lanjutan HEC-GeoRAS. Jangkauan sungai itu dan

rupa bentuk yang diperlukan bagi model HEC-RAS didigitkan dan maklumat dikeluarkan daripada satu rangkaian berceranggah segitiga (TIN) dalam Arc-GIS dan model HEC-RAS digunakan bagi keadaan semasa tanpa dinding banjir dan hasil model dikalibrasi dengan data yang dicerap. Kekangan ke atas lebar keratan rentas sungai bagi jarak sepanjang 1 km sungai dilakukan dengan tiga bentuk simulasi dinding banjir. Kaedah yang terbaik dipilih untuk digunakan bagi menambah dinding banjir di sepanjang jangkauan itu. Dalam bentuk yang pertama, lebar keratan rentas di sepanjang jarak yang sarna di sepanjang tebing sungai, iaitu 10 km, dikekang dengan dinding banjir dalam tiga jarak yang berbeza dari stesen Ahwaz, untuk menilai kesan jarak. Bentuk yang kedua melibatkan penambahan dinding yang berterusan sepanjang 30 km, bermula dari 10 km di hilir stesen Ahwaz sehingga ke penghujung jangkauan. Perubahan aras air sungai dan halaju aliran di stesen Ahwaz, disebabkan oleh panjang dinding banjir yang berlainan dan pelabagai jarak itu ditentukan dan dianalisis bagi tujuh nilai kala kembali. Pertambahan aras air sungai bagi bentuk pertama berubah daripada 0.66 hingga 1.44 m dan bagi bentuk kedua pertambahan aras air sungai mencapai 2.32 m bagi kala kembali 100 tahun. Carta yang dihasilkan boleh membantu penilaian kejuruteraan bagi teknik perlindungan banjir seumpama itu. Rangka kerja yang dihasilkan dalam kajian ini dapat digunakan sebagai satu kaedah simulasi prototaip bagi sungai lain dan boleh digunapakai bagi panjang dinding banjir yang berlainan pada sebarang jarak dari sesuatu stesen di hulu.

ACKNOWLEGEMENTS

First and foremost I am grateful to my Almighty God who helps me and eases all my difficulties and offers me every thing that I just hope to deserve for them.

I would like to express my gratitude to Associate Professor Dr. Abdul Halim Ghazali, to accept the supervisory of this work and for his invaluable guidance, suggestions and patience. I am also thankful to my committee member, Dr. Ahmad Rodzi Bin Mahmud for his attentions and comments.

There are enonnous aids that I have received from many people in Iran that I am thankful to them appreciatively. Much of the presented work would not have been done without the boundless encourage and support of my advisor Professor Mahmood Shafai-Bajestan. He was in contact all the time to advise a student from far away.

My special thanks and grateful appreciations are expressed to my mom and dad. They have supported me with their encouragements and prayers when I needed it most. I am thankful to my father who was patient on my long tenn absence at home; he motivated me to go through this work more energetic to finish it early. I thank my extremely merciful mother who prepares me to achieve every thing I want.

Special thanks are expressed to my brother, Mohammad who is my trusted ally to lean against all the time and my beloved sister Maryam.

I am grateful to my precious unique gentle husband, Sirous, whose shoulders give me a reliable back for ever. He stands a very long time far from me and just encouraged me kindly during this time. His love and supports were what made me strong in difficulties of living alone in Malaysia. His essence and peacefulness are sensed anywhere, anytime.



I certify that a Thesis Examination Committee has met on 16 August 2010 to conduct the final examination of Parisa Parvaneh on her thesis entitled "GIS-Based Modeling of the Changes in Water Level Due to Flood Walls" in accordance with the Universities and University College Act 1971 and the Constitution of the Universiti Putra Malaysia [P.U.(A) 106] 15 March 1998. The committee recommends that the student be awarded the Master of Science.

Members of the Thesis Examination Committee were as follows:

Husaini bin Omar, PhD Associate Professor Faculty of Engineering Universiti Putra Malaysia (Chairman)

Thamer Ahmad Mohammad Ali, PhD Associate Professor Faculty of Engineering Universiti Putra Malaysia (Internal Examiner)

Wan Nor Azmin b Sulaiman, PhD Associate Professor Faculty of Environmental Studies Universiti Putra Malaysia (Internal Examiner)

Zulkifli bin Yusop, PhD Lecturer Faculty of Civil Engineering Universiti Teknologi Malaysia (External Examiner)

SHAMS CODIN SULAIMAN, PhD

Professor and Deputy Dean School of Graduate Studies Universiti Putra Malaysia

Date: 26 November 2010

This thesis was submitted to the Senate of Universiti Putra Malaysia and has been accepted as fulfilment of the requirement for the degree of Master of Science. The members of supervisory committee were as follows:

Abdul Halim Ghazali, PhD

Associate Professor Faculty of Engineering Universiti Putra Malaysia (Chairman)

Ahmad Rodzi b. Mahmud, PhD

Associate Professor Faculty of Engineering Universiti Putra Malaysia (Member)

Mahmood Shafai-Bajestan, PhD

Professor Faculty of Water Science Shahid Charnran University of Iran (Member)

HASANAH MO GH

GHAZALI, PhD

Professor and Dean School of Graduate Studies Universiti Putra Malaysia

Date: 9 DEC 2010

DECLARATION

I declare that the thesis is my original work except for quotations and citations, which have been duly acknowledged. I also declare that it has not been previously and is not concurrently submitted for any other degree at UPM or at any other institutions.

PARISA PARVANEH

DATE: 16-8-2010

TABLE OF CONTENTS

| | | Page |
|------------|--|------|
| DEDICAT | ION | 11 |
| ABSTRAC | | 111 |
| ABSTRAK | | V |
| | LEDGEMENTS | VII |
| APPROVA | | IX |
| DECLARA | | XI |
| LIST OF T | | XV |
| LIST OF F | | XVI |
| | PPENDICES | XX |
| | BBREVIATIONS | XXI |
| CHAPTER | AS | |
| INT | RODUCTION | 1 |
| | | 1 |
| 1.1 | General 1.1.1 Flood | 2 |
| | 1.1.2 Effect of Flood Control Structures | 2 |
| 1.2 | | 3 |
| 1.2 1.3 | | 3 |
| | | 5 |
| 1.4 | 3 | 5 |
| 1.5 | Scope of the Study | 3 |
| 2 LIT | ERATURE REVIEW | 7 |
| 2.1 | History of Flood Damages | 7 |
| 2.2 | Necessity of Flood Investigation | 8 |
| | 2.2.1 Forecasting Future Flood Damages of Karoon River | 8 |
| 2.3 | Flood Disasters in Iran | 9 |
| 2.4 | Flood Control Methods | 13 |
| 2.5 | Structural Effects | 15 |
| 2.6 | History of Modeling Software Packages | 16 |
| 2.7 | River Simulations | 17 |
| | 2.7.1 Physical Models | 17 |
| | 2.7.2 Numerical Models in Water Engineering | 17 |
| | 2.7.3 Overview of Relevant River Simulation Studies | 19 |
| 2.8 | Selecting the Modeling Software Packages | 22 |
| 2.9 | HEC-RAS Modeling Software | 25 |
| | 2.9.1 Manning's Roughness Coefficient | 27 |
| | 2.9.2 Contraction and Expansion Coefficients | 29 |
| | 2.9.3 Calibration | 29 |
| | 2.9.4 Sensitivity Test | 30 |
| | 2.9.5 Simulation Output | 30 |
| | 0 HEC GeoRAS | 31 |
| | Modeling the Effects of Construction | 31 |
| 2.17 | 2 Geographic Information System (GIS) Application in Water | 35 |
| | Engineering | |

| | 2.13 | Flood Walls | 38 |
|---|------|--|------------|
| | 2.14 | Side Softwares in Hydraulic Modeling | 43 |
| | | 2.14.1 HYFA Statistical Software | 43 |
| | | 2.14.2 Global Mapper | 43 |
| | 2.15 | Limitations | 44 |
| 3 | MET | HODOLOGY | 46 |
| | 3.1 | | 46 |
| | 3.2 | Study Area | 46 |
| | | 3.2.1 Hydrometric Stations | 49 |
| | | 3.2.2 Reach Specification | 49 |
| | 3.3 | 1 | 50 |
| | 3.4 | • | 50 |
| | 3.5 | | 55 |
| | 3.6 | - | 56 |
| | | 3.6.1 Hydrometric Stations | 56 |
| | | 3.6.2 Groups of Data to Create River TIN | 57 |
| | | 3.6.3 Data to Create Floodplain TIN | 58 |
| | | 3.6.4 Preparing the HEC-RAS Geometric Import File | 59 |
| | | 3.6.5 Additional Geometric Data | 62 |
| | 3.7 | Flow Type and Flow Regime | 62 |
| | 3.8 | Hydrologic Data | 64 |
| | | 3.8.1 Discharges for Required Return Period | 64 |
| | | 3.8.2 Boundary Conditions | 67 |
| | | 3.8.3 Model Calibration | 68 |
| | 3.9 | River Width Restriction | 72 |
| | | 3.9.1 Raising of Floodplain Elevation | 72 |
| | | 3.9.2 Blocked Obstructions | 73 |
| | | 3.9.3 Levees | 73 |
| | 3.10 | Designs of Flood Walls Addition | 73 |
| 4 | RES | ULTS AND DISCUSSION | 75 |
| | | General | 75 |
| | | TIN from Surveyed Data | 75 |
| | 4.3 | Digitized RAS Layers | 76 |
| | 4.4 | Accuracy of TIN Extracted Data | 78 |
| | 4.5 | Hydraulic Simulation | 80 |
| | | 4.5.1 Geometric Data | 80 |
| | | 4.5.2 Results of HYFA Statistical Analysis | 82 |
| | 4.6 | Results of HEC-RAS Model Calibration | 84 |
| | 4.7 | Results of River Width Restriction Methods | 88 |
| | | 4.7.1 Raising of Floodplain Elevation | 88 |
| | | 4.7.2 Blocked Obstructions | 92 |
| | 4.0 | 4.7.3 Flood Walls in Levee Layer | 95 |
| | 4.8 | Results of Adding Flood Walls in First Design | 98 |
| | 4.0 | 4.8.1 Modifications in HEC-RAS | 99 |
| | 4.9 | River Restriction Results Effects of Flood Walls on Water Level | 101 104 |
| | | Effect of Flood Wall Distance on Water Level in the Upstream | 113 |
| | | Results of Adding 30 km Flood Walls (Second Design) | 115 |
| | 1.14 | reserve of reading so will room wants (become besign) | 113 |

| | | Changes of Flow Velocity Discussion | 119 123 |
|-------|-----|-------------------------------------|------------|
| 5 | CON | NCLUSION | 126 |
| | 5.1 | Conclusions | 126 |
| | 5.2 | Recommendations for Future Work | 129 |
| REFEI | REN | CES | 130 |
| RIODA | ТΔ | OF STUDENT | 197 |



LIST OF TABLES

| Table | | Page |
|-------|--|------|
| 2.1 | Comparison of most common natural events and their damages in Khuzestan Province (1991-2000) | 11 |
| 2.2 | Flow type classification in Khuzestan | 12 |
| 2.3 | Ranking of Khuzestan province due to flood events and casualties in years (1971-1996) | 13 |
| 2.4 | Some common nonstructural measures against flood | 14 |
| 3.1 | The characteristics of Ahwaz and Farsiat hydrometric stations | 49 |
| 3.2 | XY coordinates of one point per section | 57 |
| 3.3 | Created RAS layers in ArcMap and their descriptions | 60 |
| 4.1 | Results of data analysis by different frequency distributions | 82 |
| 4.2 | Discharges analyzed by HYFA for 2 years to 1000 years ART | 83 |
| 4.3 | Froude numbers in some cross sections resulted from HEC-RAS | 84 |
| 4.4 | Observed and computed water level at Ahwaz station for 2 years to 1000 years ART | 85 |
| 4.5 | Comparison of water level at Ahwaz station due to changes done by each method | 98 |
| 4.6 | Comparison of additional water level caused by flood walls in different distances from unstream station (10, 20 and 30 km) | 114 |

| 4.7 | Steady flow data input in HEC-RAS | 83 |
|------|--|-----|
| 4.8 | Rating curve of Ahwaz Station | 85 |
| 4.9 | Perspective plot of Karoon River in Ahwaz-Farsiat reach | 87 |
| 4.10 | Cross section of Ahwaz station after model calibration in existing condition | 87 |
| 4.11 | Cross section plot of raising the elevation of just two points | 88 |
| 4.12 | Cross section 37887.12 before raising the elevation of floodplain | 89 |
| 4.13 | Cross section 37887.12 after raising the elevation of floodplain | 89 |
| 4.14 | Cross section 36889.48 before raising the elevation of floodplain | 90 |
| 4.15 | Cross section 36889.48 after raising the elevation of floodplain | 90 |
| 4.16 | Perspective plot of river with 1 km restriction of method 1 | 91 |
| 4.17 | Profile plot of study reach before raising floodplain elevation | 91 |
| 4.18 | Profile plot of study reach after raising floodplain elevation | 92 |
| 4.19 | Blocked obstruction simulated in ArcMap | 93 |
| 4.20 | Plan view of river with blocked obstructions | 94 |
| 4.21 | Cross section of river with blocked obstructions on the river banks | 94 |
| 4.22 | Digitized flood walls in levee layer in ArcMap | 95 |
| 4.23 | Representation of flood walls in river schematic view | 96 |
| 4.24 | Representation of flood walls in cross section plot | 96 |
| 4.25 | Sequence of simulation of the restriction on cross section | 97 |
| 4.26 | Depiction the first design to add flood walls in ArcMap | 99 |
| 4.27 | Wrong recognition of the position of some flood walls in HEC-RAS | 100 |
| 4.28 | Perspective plot resulted from HEC-RAS model for phase 10 of | 101 |

| 4.29 | Perspective plot resulted from HEC-RAS model for phase 20 of adding flood walls | 101 |
|------|--|-----|
| 4.30 | Perspective plot resulted from HEC-RAS model for phase 30 of adding flood walls | 102 |
| 4.31 | Water profile plot for phase 10 | 102 |
| 4.32 | Water profile plot for phase 20 | 103 |
| 4.33 | Water profile plot for phase 30 | 103 |
| 4.34 | Comparison of water profiles for the existing condition (no flood wall) and adding first 10 kln flood walls for 100 yr return period | 104 |
| 4.35 | Comparison of water profiles for the existing condition (no flood wall) and adding second 10 km flood walls for 100 yr return period | IOS |
| 4.36 | Comparison of water profiles for the existing condition (no flood wall) and adding third 10 kln flood walls for 100 yr return period | 105 |
| 4.37 | Increased water level at Ahwaz station due to adding 10 km flood walls from 10 km downstream of this cross section | 106 |
| 4.38 | Increased water level at Ahwaz station due to adding 10 km flood walls from 20 kln downstream of this cross section | 106 |
| 4.39 | Increased water level at Ahwaz station due to adding 10 km flood walls from 30 km downstream of this cross section | 107 |
| 4.40 | Water levels at Ahwaz station due to different lengths of flood walls at the first 10 km stretch | 108 |
| 4.41 | Water levels at Ahwaz station due to different length of flood walls at the second 10 kln stretch | 109 |
| 4.42 | Water levels at Ahwaz station due to different length of flood walls at the third 10 km stretch | 110 |
| 4.43 | Effect of same length of flood walls at different distances from Ahwaz station for 10 yr return period | 111 |
| 4.44 | Effect of same length of flood walls at different distances from Ahwaz station for 100 yr return period | 111 |
| 4.45 | Effect of same length of flood walls at different distances from Ahwaz station for 1000 yr return period | 112 |

| 4.46 | Depiction the second design to add flood walls in ArcMap | 115 |
|------|---|-----|
| 4.47 | Perspective plot resulted from HEC-RAS model for 30 km continuous added flood walls in second design | 116 |
| 4.48 | Water profile plot resulted from adding 30 km continuous flood wall | 116 |
| 4.49 | Comparison of water profiles under existing condition (no flood wall) and adding 30 km continuous flood walls for 100 yr retum period | 117 |
| 4.50 | Increased water level at Ahwaz station due to adding 30 km flood walls from 10 km downstream of this cross section | 117 |
| 4.51 | Water levels at Ahwaz station due to different length of flood walls up to 30 km | 118 |
| 4.52 | Changes in average velocity along the channel due to 10 km flood walls from 10 km downstream of Ahwaz station | 119 |
| 4.53 | Changes in average velocity along the channel due to 10 km flood walls from 20 km downstream of Ahwaz station | 120 |
| 4.54 | Changes in average velocity along the channel due to 10 km flood walls from 30 km downstream of Ahwaz station | 120 |
| 4.55 | Changes in average velocity along the channel due to 30 km continuous flood walls from 10 km downstream of Ahwaz station | 121 |
| 4.56 | Changes in flow velocity at Ahwaz station and 9 cross sections at its downstream due to different lengths of flood walls up to | 121 |

LIST OF APPENDICES

| Appendix A: Simulation Result Phase of First Desi | | 140 |
|--|--|-----|
| Appendix B: Simulation Result of Phase of First Design | of Adding Flood Wall in Second gn | 150 |
| Appendix C: Simulation Result of Phase of First Design | 5 | 160 |
| Appendix D: Simulation Result Wall in Second De | of Adding 30 km Continuous Flood sign | 170 |
| Appendix E: Comparison of Res | sults | 190 |
| Appendix F: Raw Data of Annua | al Discharges | 196 |

LIST OF ABBREVIATIONS

I-D One-dimensional

2-D Two-dimensional

3-D Three-dimensional

ARI Average Recurrence Interval

DEM Digital Elevation Model

DTM Digital Terrain Model

Existing condition Condition with no flood wall

FW Flood wall

LOB Left OverBank

N Manning roughness coefficient

NS Not Significant

Phase=PH Distance from first added flood wall to

the Ahwaz station varying as 10,20 and

30km

ROB Right OverBank

RS = River station Distance from downstream end of reach

S Significant

TIN Triangular Irregular Network

Vel Total= Total velocity Average velocity of flow in total cross section

WS Water Surface

Yr Years

CHAPTER 1

INTRODUCTION

1.1 General

Flood that is caused by storms is among the most devastating natural disasters in almost any country. Although, heavy falls either in the form of rain or melting snow or ice layers are considered as the main causes for this phenomenon, it is undeniable that it can be induced by poorly managed constructions along riverine areas. Due to the development along rivers or throughout the catchments, studies are needed to simulate current and future hydrologic and hydraulic characteristics of the catchments to determine, prevent and solve the flood induced problems and also to plan future developments. Rivers under such conditions are mostly interfered by humans through changes made in their stream regime or natural bed conditions by silt removal, restricting their width and construction of different water control structures.

Hydraulic simulation of a river is necessary to detennine the rate of discharges and current and future levels of water surface profile specially after the effects of performing the river engineering designs on flood distribution at the upstream and downstream (Scott Wilson Piesold and Mahab Ghodss, 2005). Flow behavior for rivers is determined under hydrologic processes and geologic variations during different time periods. Due to natural changes as well as artificial modifications along the river and its adjacent floodplain, the behavior can always be influenced.

A category of tools utilized for hydraulic analysis of flow is mathematical modeling. These models solve the differential equations to bring about flow regime and flow characteristics as their results. Ultimately, the model outputs would be analyzed and interpreted to be applied to engineering designs (Water Research Center, 200Ib).

1.1.1 Flood

Flood is a natural disaster that should be expected to take place anywhere. If no control is imposed, it brings damages to cultivations, crops, properties, human welfare and in critical situations, it endangers human life as well. Obviously, it is impossible to prevent the damage completely. However, the purpose of floodplain management is to minimize such disasters as much as possible.

To reduce damages to properties along a river, predicting the hydraulic response of a river, such as changes in the water levels, to probable floods, has considerable importance **in** societies. **In** addition, before conducting any hydraulic structures planning on river, a reliable overview of the river response to such plans should be taken into consideration.

1.1.2 Effect of Flood Control Structures

Changes in the hydraulic parameters of a river should be considered as anthropogenic impacts. Such changes can be made in different hydraulic structures by preventing the natural flow path, storing its energy in dams and bunds, or restricting the river width by levees or flood walls. The presence of some structures

itself can affect the current water level and stream velocity of a river. These effects contribute to changes in inundated areas. The functionality of a flood control structure must be mutually satisfactory. In other words, it should reduce the severity of flood in one region meanwhile it does not increase the flood risk in other regions or at least this risk should be compensable (Lever and Daly, 2003; Remo and Pinter, 2007). Where flood control in a region brings about undesirable effect in another area, the non-structural methods for flood mitigation are proposed.

1.2 Study Area

The study area is located in the Khuzestan Province, Iran. The area is drained by Karoon River, one of the main rivers in southern Iran. It flows through Ahwaz City in its journey to the Arvand River to join Persian Gulf.

Khuzestan Province ranks second in Iran among the 28 provinces in the number of flood events in a period of 25 years with 117 events and it ranks first in damages to properties (Sabzab Arvand, 2006), even though it is believed that the reported financial losses are highly underestimated.

1.3 Problem Statement

Flood is one of the most detrimental phenomenon in Khuzestan Province, Iran. The rise of water levels in the most populated city of this province, Ahwaz, is making the damages more often. The area usually experiences many flood events either due to runoff or when the river overflow its banks.

Many studies have been done before and some structural measures had been constructed to protect the city and the river banks from flood, like designs and plans for silt removal, bunds, levees or flood walls. However, changes in river bed in a reach would affect the water level or velocity in this reach and others. It may also cause problems such as more sever floods beyond the protected reach. This effect has been implied hypothetically that the levees are expected to protect a reach from flood by conducting the additional flow to other districts (Water Research Center, 200 la; Remo and Pinter, 2007).

Confining the river cross sections by flood walls on both sides of the river downstream of Ahwaz is one of the proposed solutions against flood at riverine banks. This constriction may cause a rise in water level upstream of the protected reach, where Ahwaz City is located. Regarding the natural topography of Ahwaz which is flat, the extra elevated water levels will spread horizontally which means more areas would be flooded throughout the city. Consequently, other problems should be expected such as inundation of more roads and highways and subsequent difficulties in traffic and transportation and so on. Hence, it is vital to investigate the effects of flood walls construction on water surface profile and consider the raised water level in Ahwaz hydrometric station.

Insufficient data to run some hydraulic models, especially in a developing country such as Iran, is responsible for using improper hydraulic models which lead to inaccurate results. Thus, it is necessary to integrate a modeling approach to use the available data to run a valid model. In this area, due to lack of aerial photography and field survey which includes both floodplain and river bottom, it is not easy to run a

GIS based hydraulic modeling in which all the required geometric data can be extracted from an incessant and reliable Triangular Irregular Network (TIN) map. The approach in this thesis is to prove the capability of such discrete data to produce a complete geometric outcome and river scheme.

1.4 Objectives

The main objective of this research is to investigate the effects of flood walls on Karoon River downstream of Ahwaz City on the river water level in the city using HEC-RAS model and GIS facilities. The other specific objectives are:

- 1. To develop a method for the preparation of geometric data file for hydraulic modeling by utilizing individual topography measurements.
- 2. To assess the effect of different lengths of flood wall and its distance from Ahwaz station on the water level at this station using HEC-RAS model.
- 3. To determine the effects of cross section constriction downstream of Ahwaz city on the flow velocity before the constricted reach.

1.5 Scope of the Study

An integrated approach was introduced in this study to produce a continuous TIN in ArcMap for river bed and floodplains by using different groups of data formats for the Karoon River in Khuzestan Province, Iran. Digitizing river features and preparation of the imported file of geometric data required by hydraulic model was done by interfacing an ArcGIS extension called HEC-GeoRAS. Using these

geometric data extracted from continuous TIN and other flow data, HEC-RAS hydraulic model was run to calculate the water surface profiles along the river. This model was calibrated by comparing the simulated water levels at Ahwaz City gauge and the observed water levels at the same gauge. Floods of different return periods were computed from 44 maximum annual discharges at the gauge by HYFA software. After assessing the accuracy of the HEC-RAS model for the existing condition, flood walls were added on both sides of the river in ArcMap and were modified in HEC-RAS to constrict the width of the cross sections. This constriction was simulated in three different methods. The best method to model the considered restriction was then selected and used in all of the modeling processes in which flood walls were added in one kilometer stretch along the river in two different designs. All of the simulating processes were done repeatedly for each segment of I km flood wall to determine the water level changes at upstream gauge as a result of constriction in the widths of the river cross sections. In short, both GIS and hydraulic models were run 51 times to achieve the comparative chart of such changes. Factors such as different lengths of flood walls and different spacing between the first flood wall from the upstream gauge were studied.

REFERE CES

- Abdollahi, A., ShafaeiBejestan, M., Hasounizadeh, H. and Rostami, S. (2007). Comparing the results of HEC-RAS and MIKE 11 models in a segment of Karoon River. Paper presented at the 7th International River Engineering Conference Shahid Chamran University, Ahwaz, Iran.
- Abdolshahnejad, A. and Bahmani, B. (2002). Controlling the Erosion at Karoon River in Choneibeh Region in Ahwaz. Paper presented at the 6th International River Engineering Conference Shaid Chamran University, Ahwaz, Iran.
- Abdolshahnejad, A. and Dahanzadeh, B. (2007). The role of Bahre floodlVay in controlling water flood and stable development at the border of karoon. Paper presented at the The 7th River Engineering international Conference Shahid chamran university, Ahwaz, Iran.
- Abdovis, S. (2006). Morphological variation impacts of Karoon River at Alnvaz region on flood events. Unpublished M.Sc dessertation, Shahid Chamran University of Ahwaz, Iran.
- Abedi, M. and Hajiabadi, A. (2007). Flood Control and Training Mehranerood River. Paper presented at the 7th International River Engineering Conference Shahid Chamran University, Ahwaz, Iran.
- Abghari, H., MohseniSaravi, M., Mahdavi, M., Ahmadi, H. and azarnezhad, H. (2007). Application of hydraulic model and GIS in floodplain management. Paper presented at the 7th International River Engineering Conference Shahid Chamran University, Ahwaz, Iran.
- Aggett, G. R. and Wilson, 1. P. (2009). Creating and coupling a high-resolution DTM with a I-D hydraulic model in a GIS for scenario-based assessment of avulsion hazard in a gravel-bed river. *Geomorphology*, 113,21-34.
- Ahmad, S. and Simonovic, S. P. (2004). Spatial System Dynamics: New Approach for Simulation of Water Resources Systems. *Journal of Computilly ill Civil Engineering*. 18(4), 331-340.
- Alho, P., Russell, A. J., Carrivick, 1. L. and Ka"yhko, 1. (2005). Reconstruction of the largest Holocene jo" kulhlaup within Jo" kulsa' a' Fjo" llum, NE Iceland. *Quaternary Science Reviews*, 24, 2319-2334.
- Arico, C., Nasello, C. and Tucciarelli, T. (2009). Using unsteady-state water level data to estimate channel roughness and discharge hydrograph. *Advances in Water Resources*, 32, 1223-1240.
- Attari, A. and Abdolshahnejad, A. (2007). Examination the effects on accompolishment Plans of shore protection on the river MOIpllOlogical Changes. Case study: right hand Shore of Karoon river, choneibeh Protected arch. Paper presented at the 7th international river engineering Conference Shahid Chamran university, Ahwaz, Iran.

- Basir, A. R, Darbandi, A. S. and Jaban, E. (2007). Flood Hazard mapping with flood Depth definition in Narmab River on Golestan province. Paper presented at the 7th International River Engineering Conference Shahid Charnran University, Ahwaz, Iran.
- Bates, P. D. (2004). Remote sensing and flood inundation modeling. *Hydrological Processes*, 18,2593-2597.
- Bates, P. D., Marks, K. J. and Horritt, M. S. (2003). Optimal use of high-resolution topographic data in flood inundation models. *Hydrological Processes*. 17, 537-557.
- Bryant, R. G. and Rainey, M. P. (2002). Investigation of flood inundation on playas within the Zone of Chotts, using a time-series of AVHRR. *Remote Sensing of Environment*, 82, 360-375.
- Buttner, 0., 2007. The influence of topographic and mesh resolution in 2D hydrodynamic modeling for floodplains and urban areas. European Geosciences Union. 2007 Geophysical Research Abstracts, 9(08232).
- Carson, E. C. (2006). Hydrologic modeling of flood conveyance and impacts of historic overbank sedimentation on West Fork Black's Fork, Uinta Mountains, northeastern Utah, USA. *Geomorphology*, 75, 368-383.
- Carter, J. R. (1998). Digital representation of topographic surfaces. *Photogrammetric Engineering and Remote Sensing*, 54(II), 1577-1580.
- Casas, A., Benito, G., Thorndycraft, V. R and Rico, M. (2006). The topographic data source of digital terrain models as a key element in the accuracy of hydraulic flood modelling. *Earth Surface Processes and Landforms*. 31,444-456.
- Chang, H. H. (2004). Hydraulics and Sedimentation Studyfor the San Dieguito River Wetland Restoration Project: Environmental Services Southern California Edison Company, CA.
- Chow, V. T. (1959). Open Channel Hydraulic: McGraw-Hili Book Co, New York.
- Colby, J. D. and Dobson, J. G. (2010). Flood Modeling in the Coastal Plains and Mountains: Analysis of Terrain Resolution. *Natural Hazards Review*, 11(1), 19-28.
- Cook, A. and Merwade, V. (2009). Effect of topographic data, geometric configuration and modeling approach on flood inundation mapping. *Journal ofHydrology*, 377,131-142.
- Coroza, O., Evans, D. and Bishop, I. (1997). Enhancing runoff modeling with GIS. Landscape and Urban Planning. 38, 13-23.
- Daniari, N. and Salakhpoor, M. (2007). Evaluation of economical impacts resulting from flood damages and drought effects in dermination of legal boul/daries of

- *Karoon, Arvand and Bahmanshir.* Paper presented at the 7th International River Engineering Conference Shaid Chamran University, Ahwaz, Iran.
- Dolcine, L., Andrieu, H., Sempere-Torres, D. and Creutin, D. (2001). Flash flood forecasting with coupled precipitation model in mountainous Mediterranean basin. *Journal ofhydrologic engineering*, 6 (1), 1-10.
- Dutch, S. (2005). Converting UTM to Latitude and Longitude (Or Vice Versa). from http://www.uwgb.edu/dutchs/usefuldata/utmforrnulas.htm
- Dutta, D., Herath, S. and Musiake, K. (2000). Flood inundation simulation in a river basin using a physically based distributed hydrologic model. *Hydrological Processes*. 14,497-519.
- Ebrahimi, N. G., Davudirad, A. A. and Omidi, M. (2007). *Estimating flow characteristics in ephemeral rivers from constnlcting delay dams with HEC_RAC software*. Paper presented at the 7th International River Engineering Conference Shahid Chamran University, Ahwaz, Iran.
- EbrahimiKermani, S. and Golmaee, H. (2007a). Channel modification and its effect on river response with FLUVIAL-I2 model (Case Study: NEKA RIVER).

 Paper presented at the 7th International River Engineering Conference Shahid Chamran University, Ahwaz, Iran.
- EbrahimiKennani, S. and Golmaee, H. (2007b). Levee construction effect on river response with FLUVIALI2 model (Case Study: NEKA RIVER). Paper presented at the 7th International River Engineering Conference Shahid Chamran University, Ahwaz, Iran.
- Endreny, T. A., Wood, E. F. and Lettenmaier, D. P. (2000). Satellite-derived digital elevation model accuracy: hydrological modelling requirements. *Hydrological Processes*, 14, 177-194.
- Erhami, M. and Khosrowshahi, F. B. (2007). Sand mining pattern determination and Training schems of sand pits using GIS and HEC-RAS model. Paper presented at the 7th International River Engineering Conference Shahid Chamran University, Ahwaz, Iran.
- Erhami, M. and SalehiNeishaboori, S. A. A. (2002). *Floodplain Mapping by Using HEM-GEO Extension*. Paper presented at the 4th Hydraulic Conference, Shiraz, Iran.
- Eslamian, S. S., Mallakpour, I. and Babaahmadi, A. (2008). *Flow quality and quantity frequency analysis of river*. Paper presented at the 4th National Conference on Civil Engineering, Tehran, Iran.
- ESRI. (2008). ArcGIS (Version 9.3): New York Street, Redlands, CA.

- Faisal, I. M., Kabir, M. R. and Nishat, A. (1999). Non-structural flood mitigation measures for Dhaka City. *Urban Water*, *l*, 145-153.
- Farsi, M., Namjou, M. and Ahmadinezhad, A. (2002). River modification and Designing of Protective Wall for Halil River, Jirojt for 1000 Years Return Period. Paper presented at the 6th International River Engineering Conference Shahid Chamran University, Ahwaz, Iran.
- Fonnerly Carolina Map Distributors. (2009). Overview of Global Mapper Software. from http://www.geomart.com/products/globalmapper/overview.htm
- Galy, H. M. and Sanders, R. A. (2002). Using Synthetic Aperture Radar Imagery for Flood Modelling. *Transactions in GIS*, 6 (1), 31-42.
- Ghorbani, F., Fathi-Moghadam, M. and Bina, M. (2007). Using multiple Regression forflood modeling Case Study: Maroon, Allah and Jarahi Catchments. Paper presented at the 7th International River Engineering Conference Shahid Chamran University, Ahwaz, Iran.
- Global Mapper Blog. Global Mapper User's Manual 2009, from http://www.globalmapperforum.com/global-mapper.html
- Goodell, C. R. and Brunner, G. W. (2004). Watershed Analysis with the Hydrologic Engineering Center's River Analysis System (HEC-RAS): System-widw Modeling, Assessment, and Restoration Technologies (SMART), CA.
- Habibi, M., Arshad, S. and Hhosseini, s. (2007). *Analytical comparison of riverflow and sediment trasport models*. Paper presented at the 7th International River Engineering Conference Shahid Chamran University, Ahwaz, Iran.
- Haile, A. T. and Rientjes, T. H. M. (2005). Effects of LIDAR resolution inflood modeling: A model sensitivity studyfor the city of Tegucigalpa, Honduras. Paper presented at the 36th International Society for Photogrammetry and Remote Sensing Conference, Laser Scanning Workshop, ISPR, Enschede, The Netherlands.
- Hall, 1. W., Tarantola, S., Bates, P. and Horrill, M. S. (2005). Distributed sensitivity analysis of flood inundation model calibration. *Journal of Hydraulic Engineering*, 131(2), 117-126.
- Hamadi, K. (2003). Evaluation Discharge-Stage Relation and Investigation the continuing this values in the Big Karaon River system. Paper presented at the 6th International River Engineering Conference, Ahwaz, Iran.
- Hardy, R. J., Bates, P. D. and Anderson, M. G. (1999). The importance of spatial resolution in hydraulic models for floodplain environments. *Journal of Hydrology*, 216(1-2), 124-136.

- HEC-GeoRAS_User's Manual. (2005). HEC-GeoRAS GIS Tools for support of HEC-RAS using ArcGIS (Version 4). U.S. Army Corps of Engineers, Davis, CA.
- HEC-RAS_Hydraulic Reference. (2002). HEC-RAS River Analysis System (Version 3.1). U.S. Army Corps of Engineers, Davis, CA.
- HEC-RAS_User's Manual. (2002). HEC-RAS River Analysis System (Version 3.1). U.S. Army Corps of Engineers, Davis, CA.
- Hilldale, R. C. and Raff, D. (2008). Assessing the ability of airborne LiDAR to map river bathymetry. *Earth SUlface Processes and Landforms*. 33(5), 773-783.
- Horritt, M. S. and Bates, P. D. (2002). Evaluation of ID and 2D numerical models for predicting river flood inundation. *Journal of Hydrology*, 268, 87-99.
- Horritt, M. S. and Bates, P. D. (200 I). Predicting floodplain inundation: rasted-based modelling versus the finite-element approach. *Hydrological Processes*, 15(5), 825-842.
- Hosseini, K. (2007). River Training in urban regions (case study in Astara City, Iran). Paper presented at the 7th International River Engineering Conference Shahid Chamran University, Ahwaz, Iran.
- Hosseini, S. M. and Abrishami, 1. (2003). *Open Channel Hydraulic:* Imam Reza University, Mashhad, Iran.
- Hudson, P. F. and Colditz, R. R. (2003). Flood delineation in a large and complex alluvial valley, lower Pa'nuco basin, Mexico. *Journal of Hydrology*, 280, 229-245.
- Knebl, M. R., Yang, Z. L., Hutchison, K. and Maidment, D. R. (2005). Regional scale flood modeling using NEXRAD rainfall, GIS, and HEC-HMS/RAS: A case study for the San Antonio River Basin Summer 2002 storm event. *Journal of Environmental Management*, 75, 325-336.
- Kundzewicz, Z. M. and Schellnhuber, H. J. (2004). Floods III the IPCC tar perspective. *Natural Hazards*, 31(1), 111-128.
- Lee, K. T., Ho, Y. H. and Chyan, Y. 1. (2006). Bridge Blockage and Overbank Flow Simulations Using HEC-RAS in the Keelung River during the 200 I Nari Typhoon. *Journal of Hydraulic Engineering*, 132(3),319-323.
- Legleiter, C. 1., Roberts, D. A, Marcus, W. A and Fonstad, M. A (2004). Passive optical remote sensing of river channel morphology and in-stream habitat: physical basis and feasibility. *Remote Sensing of Environment*, 93(4),493-510.
- Lever, J. H. and Daly, S. F. (2003). Upstream Effects of Cazenovia Creek Ice-Control Structure. *Journal of Cold Regions Engineering*, 17(1),3-17.

- Lindenschmidt, K. E., Hemnann, U., Pech, I., Suhr, U., Apel, H. and Thieken, A. (2006). Risk assessment and mapping of extreme floods in non-dyked communities along the Elbe and Mulde Rivers. *Advances in Geosciences*.9, 15-23.
- Liu, Y. B., Gebremeskel, S., Smedt, F. D., Hoffmann, L. and Pfister, L. (2003). A diffusive transport approach for flow routing in GIS-based flood modeling. *Journal ofHydrology*, 283, 91-106.
- Mahdavi. (1999). *Flood Management:* Office of Investigation of Safety and Reconstruction Issues, Iran.
- Mahdavi, M., Osati, K., Sadeghi, S. A. N., Karimi, B., and Mobaraki, J. (2010). Determining Suitable Probability Distribution Models for Annual Precipitation Data (A Case Study of Mazandaran and Golestan Provinces). *Journal of Sustainable Development*. 3(1), 159-168.
- Maidment, D. R. and Djokic, D. (2000). Hydraulic and Hydrologic Modeling Support with Geographic Information System. Redlands, CA: ESRI Press.
- Marcus, W. A., Legleiter, C. J., Aspinall, R. J., Boardman, J. W. and Crabtree, R. L. (2003). High spatial resolution hyperspectral mapping of in-stream habitats, depths, and woody debris in mountain streams. *Geomorphology*, 55(1-4), 363-380.
- McKinney, D. C. and Cai, X. (2002). Linking GIS and water resources management models: an object-oriented method. *Environmental Modelling & Software.I7*, 413-425.
- Melesse, A. M. and Shih, S. F. (2002). Spatially distributed storm runoff depth estimation using Landsat images and GIS. Computers and Electronics in Agriculture, 37, 173-183.
- Merwade, V. (2009). Effect of spatial trends on interpolation of river bathymetry. Journal of Hydrology, 371, 169-181.
- Merwade, V. (2008). Tutorial on using HEC-GeoRAS with ArcGIS 9.2. School of Civil Engineering, Purdue University.
- Merwade, V., Cook, A. and Coonrod, J. (2008). GIS techniques for creating river terrain models for hydrodynamIc modeling and flood inundation mapping. *EnVironmental Modelling & Software*, 23, 1300-1311.
- Merwade, V. M., Maidment, D. R. and Hodges, B. R. (2005). Geospatial Representation of River Channels. *Journal of Hydrologic Engineering IO(3)* 243-251.
- Methods, H. Dyhouse, G., Hatchett, J. and Benn, J. (2003). Floodplain Modeling using HEC-RAS: Haestad Press, Waterbury, CT.

- Minnerly, B. (2006). River elevation modeling: an integrated HEC-RAS *ArcG1S* approach. Retrieved from *http://google.* unm.edu/search?q=Final+project%3A+CE+547%2C+spring%2 C+2006+Blake+Mmnerly&sIte=UNM&c1lent=UNM&proxystylesheet=UN *M&output=xml_*no_dtd&submit.x=17&submit.y=3
- Motiei, H. (2004). Application of Geographic Information System (GIS) in Flood River Engineering: Iranian National Committee on Large Dams, Tehran, Iran.
- Motiei, H. (2005). *Knowing to ArcView-GIS and Extensions* (Vol. I): University of *Water* and Power (Shahid Abbaspour), Tehran, Iran.
- Musavi-jahromi, H. and Taghizade, K. (2007). *The effect of shape and numbers of bridge piers on scour*. Paper presented at the *7th* International *River* Engineering Conference Shahid Chamran University, Ahwaz, Iran.
- Naziri, A. S. (2007). Flood Hazard Mapping for Sungai Benus Catchment using HEC-RAS and Arc View GIS. Unpublished M.Sc dessertation, Universiti Putra Malaysia.
- Nicholas, A. P. and Walling, D. E. (1997). Modelling flood hydraulics and overbank deposition on *river* floodplains. *Earth Surface Processes and Landforms*, 22, 59-77.
- Noman, N. S., Nelson, E. 1. and Zundel, A. K. (2001). Review of the automated floodplain delineation from digital terrain models. Journal of Water Resources Planning and Management, 127(6),394-402.
- Norouzi, B. and Barani, G. A. (2007). Flood Spread Analysis in River Floodway Using Hec-Rass Software. Case Study (The Successive Floods of Gales/an Provience). Paper presented at the 7th International River Engineering Conference Shahid Chamran University" Ahwaz, Iran.
- Office of Standars_Water Resource Management. (2005). Guide to De/ermine the Return Period for River Engineering Projects. Iran.
- Omer, C. R., Nelson, E. 1. and Zundel, A. K. (2003). *Impact* of varied data resolution on hydraulic modeling and floodplain delineation. *Journal of the American Water Resources Association*, 39(2),467-475.
- Pappenberger, F., Beven, K. J., Ratto, M. and Matgen, P. (2008). Multi-method global sensitivity analysis of flood inundation models. Advances in Water Resources, 31, 1-14.
- Parchure, T.M., 2005. Structural Methods *to* Reduce Navigation Channel Shoaling: US Anny Corps of Engineers, *Engineer* Research and Development Center, ERDC/CHL TR-05-13, 77p.

- Patro, S., Chatterjee, *C.*, Singh, R. and Raghuwanshi, N. S. (2009). Hydrodynamic modelling of a large flood-prone river system in India with limited data. *Hydrological Processes*, 23,2774-2791.
- PoorrezaBilondi, M., Kahe, M., and Kashefipour, M. (2007). Determination of floodplain in Great Karoon River (Molasani -Ahwaz) using of HEC-RAS and HEC-GeoRAS softwares. Paper presented at the 7th International River Engineering Conference Shahid Chamran University, Ahwaz, Iran.
- Pourabadeh, T., Fasihi, M. and Dallalzadeh, A (2007). Application of soft wares in determine of riverbed and right of rivers, Case study; Shoor River (Zayandehrood basin). Paper presented at the 7th International River Engineering Conference Shahid Chamran university, Ahwaz, Iran.
- Raber, G. T., Jensen, J. R., Hodgson, M. E., Tullis, J. A, Davis, B. A and Berglund, J. (2007). Impact of LIDAR nominal post-spacing on DEM accuracy and flood zone delineation. *Photogrammetric Engineering and Remote Sensing*, 73 (7), 793-804.
- Refsgaard, 1. C. and Henriksen, H. J. (2004). Modelling guidelines e terminology and guiding principles. *Advances in Water Resources*, 27(1), 71-82.
- Remo, J. W. F. and Pinter, N. (2007). Retro-modeling the Middle Mississippi River. Journal of Hydrology, 337,421-435.
- Robayo, O. and Maidment, D. (2005). Map to Map: Converting a NEXRAD Rainfall Map into a Flood Inundation Map. Austill: Center for Research in Water Resources-The University of Texas at Austin.
- Sabzab Arvand. (2006). Classification different regions of Khuzestan Province in aspect ofphenomena events. Ahwaz, Iran.
- Sadrolashrafi, S. S. (2008). Flood Modeling Using WMS Software: A Case Study of Dez River Basin, Iran. Umiversiti Putra MalaySia.
- Saqhafian, B., Jabbari, E., Mosaferi, M. and Danaiean, M. R. (2002). Combining of Numerical Models and GIS for Flood Mapping and Damage Estimating. Paper presented at the 4th Hydraulic Conference, Shiraz, Iran.
- Sauer, A. N. and O'Neill, P. A. (2001). Stormwater Master Planning with GIS: A Case Study of the Blue River Watershed. Retrieved 2007, from http://proceedings.esri.com/Iibrary/userconf/procOI/professional/papers/pap9 46/p946.htm
- Scott Wilson Piesold and Mahab Ghodss. (2005). Management of Karoon River System. Ahvaz, Iran.
- Shahinejad, B., Zahiri, A and Rostami, S. (2008). Prediction of erosion and sedimentation in Karoun river in vicinity of Ahwaz using GSTARS

- *mathematical model.* Paper presented at the 4th international congress of Civil engineering, Tehran University, Tehran, Iran.
- ShehniDarabi, B. and Moradi, M. (2008). *Calculation of the flood return period at Karoon III Dam*. Paper presented at the 4th National Civil Symposium, Tehran, Iran.
- Sheng, Y., Gong, P. and XIiao, Q. (200 I). Quantitative dynamic flood monitoring with NOAA AVHRR. *International Journal of Remote Sensing*, 22, 1709-1724.
- Singh, A, Fosnight, E. A, Smith, J. S., Emste, M., Giese, K., Shi, H., Singh, R. and Hossain, N. (2002). Early Warning, Forecasting and Operational Flood Risk Monitoring in Asia (Bangladesh, China and India): UNEP-Division of Early Warning & Assessment, Nairobi, Kenya.
- Sinnakaudan, S. K., Ghani, A. A., Ahmad, M. S. S. and Zakaria, N. A. (2003). Flood risk mapping for Pari River incorporating sediment transport. *Environmental Modelling & Software*, 18, 119-130.
- Smemoe, C. M., Nelson, E. J., Zundel, A. K. and Miller, A. W. (2007). Demonstrating Floodplain Uncertainty Using Flood Probability Maps. Journal of Water Resources Assodation, 43 (2), 359-371.
- Smith, K. and Ward, R. (1998). Floods: Physical processes and human impacts. West Sussex, England: John Wiley & Sons Ltd.
- Sugarcane Development. Flood protection: Karab Cunsulting Engineers, Ahwaz, Iran.
- Tabatabaei, A R. (2007). Investigation and Application of RiverCAD River Modeling Software Package in River Engineering. (Case Study: Pelasgan River in Isfahan). Paper presented at the 7th International River Engineering Conference Shahid Chamran UlliVerslty, Ahwaz, Iran.
- Tate, E. and Maidment, D. (1999). Floodplain Mapping Using HEC-RAS and ArcView GIS. Austin: Bureau of Engineering Research The University of Texas at Austin.
- Tate, E. C., Maidment, D. R., Olivera, F. and Anderson, D. J. (2002). Creating a Terrain Model for Floodplain Mapping. *Journal ofHydrologic Engineering*, 7(2), 100-108.
- Thoms, E. (2005). Creating and Managing Digital Geologic Cross Sections within ArcGIS. Retrieved from http://pubs.usgs.gov/ofl20051l428/thoms/index.htmIU.S. Geological Survey 4200 University Ave. Anchorage, AK
- Todini, E. (1999). An operational decision support system for flood risk mapping, forecasting and management. *Urban Water*, 1(2), 131-143.

- Trigg, M. A., Wilson, M. D., Bates, P. D., Horritt, M. S., Alsdorf, D. E., Forsberg, B. R, et al. (2009). Amazon flood wave hydraulics. *Journal of Hydrology*, 374, 92-105.
- Tucci, C. E. M. and Villanueva, A. O. N. (1999). Case study Flood control measures in Uniao da Vitoria and Porto Uniao: structural vs. non-structural measures. *Urban Water,i*, 177-182.
- US Army Corps of Engineers (USACE), 1993. River Hydraulics. US Army Corps of Engineers, Washington, DC.
- US Army Corps of Engineers (USACE) 2004. Upper Mississippi RiverSystem flow frequency study. http://www.mvr.usace.army.miVpdw/pdf/FlowFrequency/Documents/FinaIReport/default.asp.
- Vatanfada, J. (2004). Flood Management in IRAN. Ministry of Energy Water Resources Management Organization. Deputy of Operation and Conservation, Head of flood control, river and coastal engineering bureau.
- Vidal, J. P., Moisan, S. and Faure, J. B. (2003). Knowledge-Based Hydraulic Model Calibration. In V. Palade, R. 1. Howlettand L. C. Jain (Eds.), *Knowledge-Based Intelligent Information and Engineering Systems* (pp. 676-683). Verlag Berlin Heidelberg: Springer.
- Vidal, 1. P., Moisan, S., Faure, 1. B. and Dartus, D. (2007). River model calibration, from guidelines to operational support tools. *Environmental Modelling & Software*, 22, 1628e1640.
- Wadsworth, G. (1999). Flood Damage Statistics. Public Works Department, Napa, CA.
- Water Research Center. (200Ia). *Investigation of Modification Design of KaroOll River_Flood Studies*. Ahwaz, Iran.
- Water Research Center. (2001b). Investigation of Modification Design of Karoon River_Hydraulic Studies. Ahwaz, Iran.
- White, L. and Hodges, B. R. (2005). Filtering the Signature of submerged large woody debris from bathymetry data. *Journal ofHydrology*, 309(1-4), 53-65.
- Wilson, M., Bates, P., Alsdorf, D., Forsberg, B., Horritt, M., Melack, J., et al. (2007). Modeling large-scale inundation of Amazonian seasonally flooded wetlands. *Geophysical Research Letters*, 34(15).
- Yochum, S. E., Goertz, L. A. and Jones, P. H. (2008). Case Study of the Big Bay Dam Failure: Accuracy and Comparison of Breach Predictions. *Journal of Hydraulic Engineering*. 134(1), 1285-1293.
- Zarrin, H., Vafakhah, M., Namdorost, J. and Nahtani, M. (2006). *Attributes offloods of Karoon River in Ahwaz region*, Shahrekord, Iran.